

CITY OF NEWPORT BEACH WATERFRONT PROJECT GUIDELINES AND STANDARDS

HARBOR DESIGN CRITERIA COMMERCIAL & RESIDENTIAL FACILITIES



2017 EDITION

DRAFT, September 1, 2020



- (3) Fabrication and erection shall comply with the latest applicable codes as noted:
 - (a) AISC, Latest Editions
 - (b) Aluminum Structural Welding Code, Latest Edition
 - (c) Aluminum Design Manual, Latest Edition
 - (d) 12011 Design Manual for Structural Stainless Steel, Latest Edition
- (4) All bolts securing primary structural members shall be a minimum of ½ inch diameter thru bolts. Bolts shall be minimum A307 and include washers where direct contact with timber members occurs. Carriage bolts are also allowed.
- (5) No connecting device shall protrude beyond the fascia or waler into the berthed area, which may contact any part of the berthed vessel, or extend up into any walking surface creating a tripping hazard.
- d. Concrete and Reinforcing:
 - (1) Concrete shall be designed for permeability, strength, chemical stability and abrasion resistance, appropriate for its application. Minimum compressive strength for concrete, subject to salt water splash, immersion and/or brackish water is 5,000 psi and a 0.4 water-to-cement ratio.
 - (2) In absence of soil report recommendations based on soil testing, Portland cement shall conform to ASTM C 150 Type-I or Type II concrete meeting Exposure classification S1 and having minimum f'c=4000 psi and a water cement ratio of 0.4 shall be used in compliance with ACI -14 Table 19.3.2.1, modified, and low alkali. Chemical admixtures shall conform to ASTM C 494. Chemicals designed to limit corrosion of internal reinforcing may be used. Air entrainment admixtures shall conform to ASTM C 260. Coarse and fine aggregate shall conform to ASTM C 33, and ASTM C 330 where lightweight aggregates are used. Lightweight aggregate, if used, shall consist of expanded and coated shale or equivalent material of sufficient strength and durability to provide concrete of the required strength.
 - (3) Concrete structures shall be designed to provide sufficient coverage of reinforcing steel, so as to prevent corrosion, per code

Redline

requirements. For structures exposed to salt water splash or immersion, bar reinforcement shall conform to ASTM A 706, and shall be epoxy coated per ASTM A-<u>775 orand ASTM A-</u>934, after bending of the bars. Welded wire mesh shall conform to ASTM A 185 and shall be <u>galvanized or</u> epoxy coated conforming to ASTM A 884, with all visible defects and cut ends repair coated. Wires used to tie reinforcing steel shall be either epoxy-coated steel, or 316 stainless steel.

- e. Pilings and Anchorage:
 - (1) Piles shall be the products of manufacturers and contractors regularly engaged in the production of such items for marine construction. Typical materials approved for pile materials include: 1. Pre-stressed concrete, 2. Steel, or 3. High-strength composite materials. Timber piles are not allowed.
 - (2)Unless subsurface soil materials prevent their use, pilings shall be pre-stressed concrete. In absence of soil report recommendations based on soil testing, Portland cement shall be ASTM C150 Type II₂ concrete meeting Exposure classification S1 and having minimum f'c=4000 psi and a water cement ratio of 0.4 shall be used in compliance with ACI -14 Table 19.3.2.1, modified, low alkali. Water for mixing and curing shall be fresh, clean and potable. Aggregates shall conform to ASTM C33, Size Number 67, and be free from any substance that is deleteriously reactive with the alkalis in the cement. Admixtures, if used, shall conform to the requirements of ASTM C494 and not contain chlorides. Corrosion inhibiting concrete admixtures are encouraged. Prestressing steel shall be uncoated, seven-wire stress relieved strand with a minimum ultimate stress of 270,000 psi conforming to ASTM A416. Ties and spirals shall conform to ASTM A82, cold drawn and shall be epoxy coated per ASTM A775 or ASTM A934. Piles shall cure and reach a strength of not less than 4,000psi before de-tensioning and cutoff of the strands.
 - (3) Guide rollers shall be fabricated from polyethylene, UHMW, polyolefin or polyurethane roller or plate material. As an option to the use of rollers, UHMW rub blocks may be used. Minimum thickness of a rub block shall be 2 inches, with attachment bolts countersunk into the UHMW material.
 - (4) Any structural steel components used in the marine environment shall be hot-dip galvanized or epoxy coated per manufacturers recommendations, or 316 stainless steel.



Cantilevered seawalls are limited by the height of the wall above the waterside mudline and are generally effective for exposed heights of not more than 8 feet. For structural steel type seawalls, higher exposed heights are possible. Tied-back seawalls can be effective for exposed heights over 8 feet and may require continuous caps, walers (beams), steel tie rods and a foundation anchors (Deadman), or earth anchors. Tie-back anchor systems shall require protection against corrosion. Galvanic anode cathodic protection system is recommended. Tie-back anchor system shall be designed to last the life of seawall.

- (2) Generally, seawall sheets constructed of reinforced, prestressed concrete are desirable, although for special conditions, structural steel interlocking sheets may be necessary. Steel products in the marine environment require special non-toxic coating protection and cathodic protection, in order to provide extended life spans.
- (3) Seawalls shall have a top elevation of not less than 10.0 MLLW. Seawall elevations of greater than 10.0 MLLW may be required by City of Newport Beach, depending on location within the bay and potential for wave or wake over-topping.

The City understands there is a threat of flooding and inundation in and around Newport Harbor due to sea level rise. Newport Harbor and adjacent low-lying areas rely on a system of harbor bulkheads, seawalls, revetments, or other improvements to function. Additionally, the system of shoreline defenses protects existing development, public access, public views, and scenic qualities of the coastal zone. The City is committed to using the best-available science to determine a range of sea level rise projections for use in developing harbor development standards and in reviewing coastal development permit applications. Currently the best available science is California Coastal Commission Sea Level Rise Policy Guidance Final Adopted Science Update, November 7, 2018. Experts acknowledge that there is considerable uncertainty in the magnitude and rate of the rise in sea level and as a result, the City recognizes the need to set standards over time that provide protection from future sea level rise given the inherent uncertainty of the rate and magnitude of the rise.

Bulkheads, seawalls or other protective improvements shall be constructed and maintained as shown in Table No. 2. The structure shall also be initially designed and constructed to accommodate and receive future increases in height when directed by the City to protect adjacent low-lying areas from future flooding without the need to replace or substantially alter the structure.

Redline

Table No. 2

Year Structure Permitted	Initial Top of Structure Elevation ¹		Ultimate Design Height Elevation ²	
	NAVD88	MLLW	NAVD88	MLLW
2020-2024	<u>10.7</u>	<u>10.9</u>	<u>13.7</u>	<u>13.9</u>
2025-2029	<u>10.9</u>	<u>11.1</u>	<u>14.4</u>	<u>14.6</u>
2030-2034	<u>11.0</u>	<u>11.1</u>	14.6	14.8

- 1. The initial top of Structure elevations are derived using the Low Risk Aversion probabilistic sea level rise protection scenario for the Los Angeles tidal gauge estimated 75 years into the future based on the California Coastal Commission Sea Level Rise Policy Guidance Final Adopted Science Update, November 7, 2018. This scenario accounts for the upper range of what is "likely to occur" with approximately a 17 percent probability that sea level rise exceeds the elevations shown.
- 2. The ultimate design height elevations are derived using the Medium-High Risk Aversion probabilistic sea level rise protection scenario for the Los Angeles tidal gauge, again estimated 75 years into the future based on the California Coastal Commission Sea Level Rise Policy Guidance Final Adopted Science Update, November 7, 2018. This scenario accounts for increased sea level with approximately a 1-in-200 or 0.5 percent probability that sea level rise exceeds the elevations shown.
 - (4) The distance between seawalls and all floating dock components shall be a minimum of one foot horizontal distance.
 - (5) Seawalls shall be designed to resist all applicable vertical and horizontal loads.
 - (6) A minimum safety factor of 1.5 shall apply to gravity loads, and a minimum safety factor of 1.1 shall apply to seismic loading cases for the stability of seawalls.
 - (7) Decking may but to the seawall cap, or cantilever over the top of the seawall, if approved.
 - (8) Wing Walls: Wing walls are retaining walls that project landward and are perpendicular to the seawall. Wing walls may be necessary to isolate the seawall protection system of one property to the adjacent property. Special care must be taken to assure that the construction of a seawall for the subject property does not adversely impact the seawalls of the adjoining properties, either during construction or over the life of the structures.





- (1) Pierhead and Project lines are established and managed by the federal government.
- (2) Portions of floating docks that extend into Federal Dredging Limit must be removed by the dock owner at dock owner's cost, when the federal government conducts periodic dredging operations.
- (3) Dock fingers must be designed and constructed in such a way that provides structural integrity of the fingers to resist lateral impact and mooring line loads without guide piles at the finger ends. Continuous structural stringers and/or special moment and shear-carrying splices must be engineered and submitted to the Building Department for approval.
- (4) The City Council has allowed exceptions for dock construction beyond the Pierhead Lines as noted in Council Policy H-1.

DOCK ARRANGEMENT CASE 9B





















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DOCK CLEAT (TYPICAL DOCK MOORING CLEAT)				
 NOTES: EYE BOLT SHALL BE FABRICATED OF STAINLESS OR GALVANIZED STEEL. EPOXY SHALL CONFORM TO ASTM C881 STANDARD, OR SHALL BE AN EPOXY FORTIFIED GROUT INTENDED TO BE USED FOR PERMANENT ANCHORAGE OF EQUIPMENT, AND SHALL BE APPLIED IN ACCORDANCE WITH THE MANUFACTURER'S WRITTEN INSTRUCTIONS. DOCK CLEATS SHALL BE GALVANIZED CAST METAL WITH HEX-HEAD THRU-BOLTS CONNECTED INTO THE TIMBER FRAMING WITH WASHER AND NUT, ALL GALVANIZED. DIAMETER OF THRU-BOLT PER CLEAT MANUFACTURER RECOMMENDATIONS. CLEAT AND BOLT SIZES BY DESIGN ENGINEER. 				
APP				
CITY OF NEWPORT BEACH DEPARTMENT OF PUBLIC WORKS				
DRAWN: P. KHARAZMI EYE BOLT OR CLEAT				
DATE: 1/8/2020 FOR BOAT ANCHORAGE 611 SHEET 1 OF 1				



TIDAL DATUMS AND ELEVATIONS



APP.