# Attachment No. PC 4 Project Plans

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# SBRANDON ARCHITECTS, INC.





# **ZENK RESIDENCE**

2830 BAYVIEW DR., CORONA DEL MAR, CA 92625

PROJECT THE	NET. STRUCTURAL DRAWNIGS FOR SPICIAL INSPECTIONS BY ENGINEER OF RICORD     HERS VERPICATION REQUIRED, REF. ENERGY CALCULATIONS	A.C.A.GOMA FERMIT IS REQUIRED FOR DICAMATIONS DEEPER THAN 57 AND FOR SHORMON AND UNDERPRINNING, CONTRACTOR TO PROVIDE A COPY OF GOMA FRAME.  2. CONTRACTOR OF GOMA FRAME.  2. CONTRACTOR SHALL USE THE CITY STRAIGHARD FORM "30-DAY NOTICE OF INTER!" TO EXCLUSITE TO MOTIFY ADMICISTLY OWNERS MY CITY HIS DIME, 30-05 SHORMON DESCRIPTION OR SHORMON. CITY STRAIGHARD FORM CAN BE CRITINALD AS:  WITTENSOWN ASSEMBLES CALL CONTROLS STRAIGHARD FORM "30-DAY NOTICE OF STRAIGHARD FORM CAN BE CRITICALD AS:  WITTENSOWN ASSEMBLES CALL CONTROLS STRAIGHARD FORM "30-DAY NOTICE OF STRAIGHARD FORM CAN BE CRITICALD AS:  WITTENSOWN ASSEMBLES CALL CONTROLS STRAIGHARD FORM "30-DAY NOTICE OF STRAIGHARD FORM CAN BE CRITICALD AS:  WITTENSOWN ASSEMBLES CALL CONTROLS STRAIGHARD FORM "30-DAY NOTICE OF STRAIGHARD FORM CAN BE CRITICALD AS:  WITTENSOWN ASSEMBLES CALL CONTROLS STRAIGHARD FORM "30-DAY NOTICE OF STRAIGHARD FORM CAN BE CRITICALD AS:  WITTENSOWN ASSEMBLES CALL CONTROLS STRAIGHARD FORM "30-DAY NOTICE OF STRAIGHARD FORM CAN BE CRITICALD AS:  WITTENSOWN ASSEMBLES CALL CONTROLS STRAIGHARD FORM "30-DAY NOTICE OF STRAIGHARD FORM CAN BE CRITICALD AS:  WITTENSOWN ASSEMBLES CALL CONTROLS STRAIGHARD FORM "30-DAY NOTICE OF STRAIGHARD FORM CAN BE CRITICALD AS:  WITTENSOWN ASSEMBLES CALL CONTROLS STRAIGHARD FORM "30-DAY NOTICE OF STRAIGHARD FORM	DIG ALER: 1.80.277.200 SUMPRIN CALIFORNA EDISOR SUMPRIN CALIFORNA EDISOR SUMPRIN CALIFORNA EDISOR SUMPRIN CALIFORNA EDISOR SUMPRIN CALIFORNA EDIS CITY MARTIR SERVER: 1.888.444.201 FINCING BELL'ELEPHONE: 1.800.750.2365 CITY OR REWORD BELOF FINCING WERSON BELOF T. CITY OR REWORD BELOF PLANNING DEFT: CITY OR REWORD BELOF PLANNING DEFT: CITY OR REWORD BELOF BELOF READ BELOF THE SERVER T	ARCHITECT BRANDON ARCHITECTS INC. 151 ALMANE SIDNES SOIT E-1 151 ALMANE SIDNES SIDNES SOIT E-1 151 ALMANE SIDNES SIDNES SOIT E-1 151 ALMANE SIDNES
10 VICINITY MAP	8 SPECIAL INSPECTIONS	6 SHORING & EXCAVATION	4 AGENCIES & SERVICES	2 PROJECT DIRECTORY
THE SUBJECT PROPERTY'S LOCATED IN NEWFORT BACH, CA N THE AREA MOWN AS CORDINA GEL MARE SOUTHWAST OF PACIFIC COAST HORIWAY. THE LOTS A PROPORABILITY OF THE SET APPLY SOUTH BY 1990 BHY	1. LANDOCAPE PLACE BOD, FIRE PIT, ACCESSION'S STRUCTURES, MACRIMEY OR CONCRETE WALLSFERKES, RETAINING WALLS OVER 4 FT. HIGH FROM THE BOTTOM OF THE COMMANDIOR TO THE FOR WALLL WIGHER SEPARATE REVIEWS AND PRIMITS SCR HO.A. PURPOSESS.  2. POINTS, SPAIN, WALLS, FENCES, PATO COMPERS AND OTHER FRESTANDING STRUCTURES SEQUIPE SEPARATE REVIEWS AND PRIMITS. THE STRUCTURE SPAIN WALLS, FENCES, PATO COMPERS AND OTHER FRESTANDING STRUCTURES SEQUIPE SEPARATE REVIEWS AND PRIMITS. THE GOAD WITH THEM AND RECENTIONS CONSENTY, LOCATION OF MEASUREMENT TO BE AT ADMICTIC FORMAN AND PRIMITS.  4. FIRE SPRINGLES SEQUIPED, CONTINUED TO SIGNATURE SEPARATE ROBBINGTO FOR ADMICTICS AND PROPERTY PRITO OR POPULATION.  5. AUDITOR OF WITH TO SERVING STRUCTURE SPAIN EXPENSE TO MALE TO ADMICTICS AND PROPERTY OR TO SERVE AND PRIMITS.  6. PO SOURCE STRUCTURE OF BUILDINGS WORKING AT THE BUILDINGS FROM SOCIETATION BE SERVICULATED THE SERVICE STRUCTURE SERVICE STRUCTURE. SERVICE STRUCTURE STRUCTURE STRUCTURE STRUCTURE STRUCTURE STRUCTURE STRUCTURE.  6. PO SOURCE STRUCTURE STRUCTURE STRUCTURE STRUCTURE STRUCTURE STRUCTURE.  6. PO SOURCE STRUCTURE STRUCTURE STRUCTURE STRUCTURE STRUCTURE STRUCTURE.  6. PO SOURCE STRUCTURE STRUCTURE STRUCTURE STRUCTURE STRUCTURE STRUCTURE.  6. PO SOURCE STRUCTURE STRUCTURE STRUCTURE STRUCTURE STRUCTURE.  6. PO SOURCE STRUCTURE STRUCTURE STRUCTURE STRUCTURE.  6. PO SOURCE STRUCTURE.  6.	BUILDING AREA SCHEDULE  NAME  3 CAR GRAME  707 SF  SURTOTAL GRAME  707 SF  707 SF  708 ST  708 ST  709	LEGAL RECORPTOR  APPR IDEALTY OF PARKE STATE OF CALFFORM  PORTHAGE OF LOT 1.3.5 & LOT 7.4 & LI N RLOCK 13.4 CITY OF CORDINA  DOSING REMANDE OF TO SHAME, STATE OF CALFFORM  DOSING REMANDER OF TO SHAME, STATE OF CALFFORM  DOSING REMANDER OF TO SHAME STATE OF CALFFORM  DOSING REMANDER OF THE OFFICE OF THE CALFFORM  CONTROL REMANDER OF THE STATE OF THE CALFFORM  CONTROL REMANDER OF THE STATE OF THE CALFFORM OF THE STATE OF THE CALFFORM OF THE STATE OF THE CALFFORM OF THE CALFFO	CODES:  NRMC_PROPORT ESCAP MANUEPPAL CODE; 2002 CDC_2002 CDC_2002 CDC_AND 2002 CDC_2002 CDC_AND 2002 CDC_2002 CDC_AND 2002 CDC_2002 CDC_AND 2002 CDC
11 PROJECT DESCRIPTION	9 DEFERRED SUBMITTALS	7 PROJECT STATISTICS	5 SITE INFORMATION	3 CODE INFORMATION

BRANDON ARCHITECTS PROJECT STATUS
VARIANCE/CDP PLAN CHECK NO. PA2024-0057 PROJECT CONTACT GABBY UVIDIA ZENK RESIDENCE OWNER INFORMATION: JASON ZENK 235 HELIOTROPE AVE., CORONA DEL MAR, CA, 92825 TITLE SHEET T-1.0 61 1 SHEET LIST

T-1.1

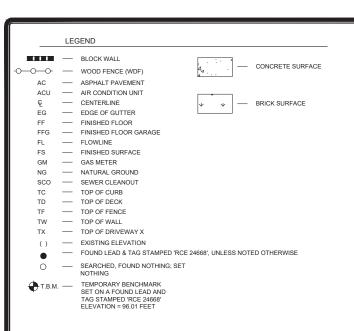
C

ABBREVIATION

B GENERAL NOTESON

DIL-GAS-RETARDER: A MINIMUM 6-MIL (0.15 MM) [OR 3-MIL (0.075 MM) CROSS-LAMINATED] POLYETHYLENE OR FXIBI F SHEFTING MATERIAL SHALL BE PLACED ON TOP OF THE GAS-PERMEABLE LAYER PRIOR TO CASTING THE

A CONTRACT DOCUMENT NOTES



TITLE REPORT/EASEMENT NOTES

2830 BAYVIEW DRIVE CORONA DEL MAR, CA 92625

NO TITLE REPORT PROVIDED

LEGAL DESCRIPTION

REAL PROPERTY SITUATED IN THE CITY OF NEWPORT BEACH, COUNTY OF ORANGE, STATE OF CALIFORNIA AND IS DESCRIBED AS FOLLOWS:

PARCEL 1:

THE SOUTHEASTERLY 59 FEET OF LOTS 1, 3, 5, AND 7, THE SOUTHEASTERLY 59 FEET OF LOTS 1, 3, 5, AND 7, EXCEPTING THE NORTHEASTERLY 10 FEET OF LOT 7, ALL IN BLOCK 134, RESUBDIVISION OF CORONA DEL MAR, IN THE CITY OF NEWPORT BEACH, COUNTY OF ORANGE, STATE OF CALIFORNIA AS PER MAP RECORDED IN BOOK 4, PAGE 67, OF MISCELLANGOUS MAPS IN THE OFFICE OF THE COUNTY RECORDER OF SAID COUNTY.

PARCEL 2:

AN EASEMENT FOR PUBLIC UTILITY PURPOSES OVER THE NORTHEASTERLY 5 FEET OF THE SOUTHWESTERLY 15 FEET OF THE NORTHWESTERLY 59 FEET OF LOT 7 IN BLOCK 134, RESUBDIVISION OF CORONA DEL MAR PER MAP RECORDED IN BOOK 4, PAGE 67, OF MISCELLANEOUS MAPS IN THE OFFICE OF THE COUNTY RECORDER OF SAID COUNTY

EXISTING BUILDING

### BENCHMARK INFORMATION

BENCHMARK NO: M-782

DESCRIBED BY OCS 2002 - FOUND 3 3/4" OCS ALUMINUM BENCHMARK DISK STAMPED "M 782 1946", SET IN THE TOP OF A CONCRETE BRIDGE ABUTMENT. MONUMENT IS LOCATED IN THE SOUTHWEST CORNER OF THE INTERSECTION OF GOLDENROD AVENUE AND THE CONCRETE PEDESTRIAN BRIDGE OVER BAYSIDE DRIVE, 6 FT. NORTHWESTERLY OF THE CENTERLINE OF THE BRIDGE AND 1 FT. SOUTHEAST OF THE NORTHWEST END OF THE SOUTHWEST ABUTMENT. MONUMENT IS SET 0.5 FT. BELOW THE BRIDGE SIDEWALK.

ELEVATION: 95.032 FEET (NAVD88), YEAR LEVELED 2011

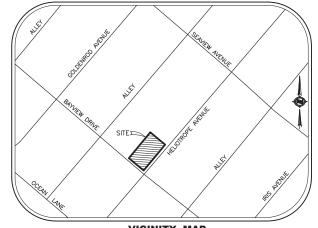
POR. OF

THE BEARINGS SHOWN HEREON ARE BASED ON THE CENTERLINE OF HELIOTROPE AVENUE HAVING A BEARING OF N40°32'24"E PER CR 2018-0357

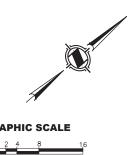
	BLE	
LINE	BEARING	DISTANCE
L1	N41°28'56"E	4.97'

POR. OF

LOT 7



VICINITY MAP



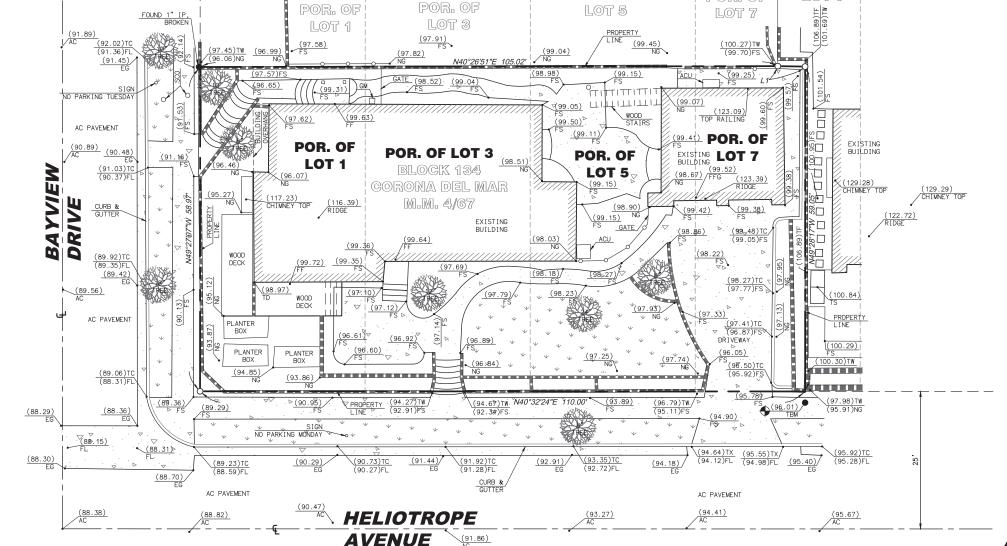
9 €

D SURVEYING INC.

STATE OF THE PARTY OF THE PARTY

APEX

**GRAPHIC SCALE** ( IN FEET ) 1 INCH = 8 FT.



POR. OF

PAUL D. CRAFT, P.L.S. 8516 LICENSE RENEWAL DATE 12/31/24

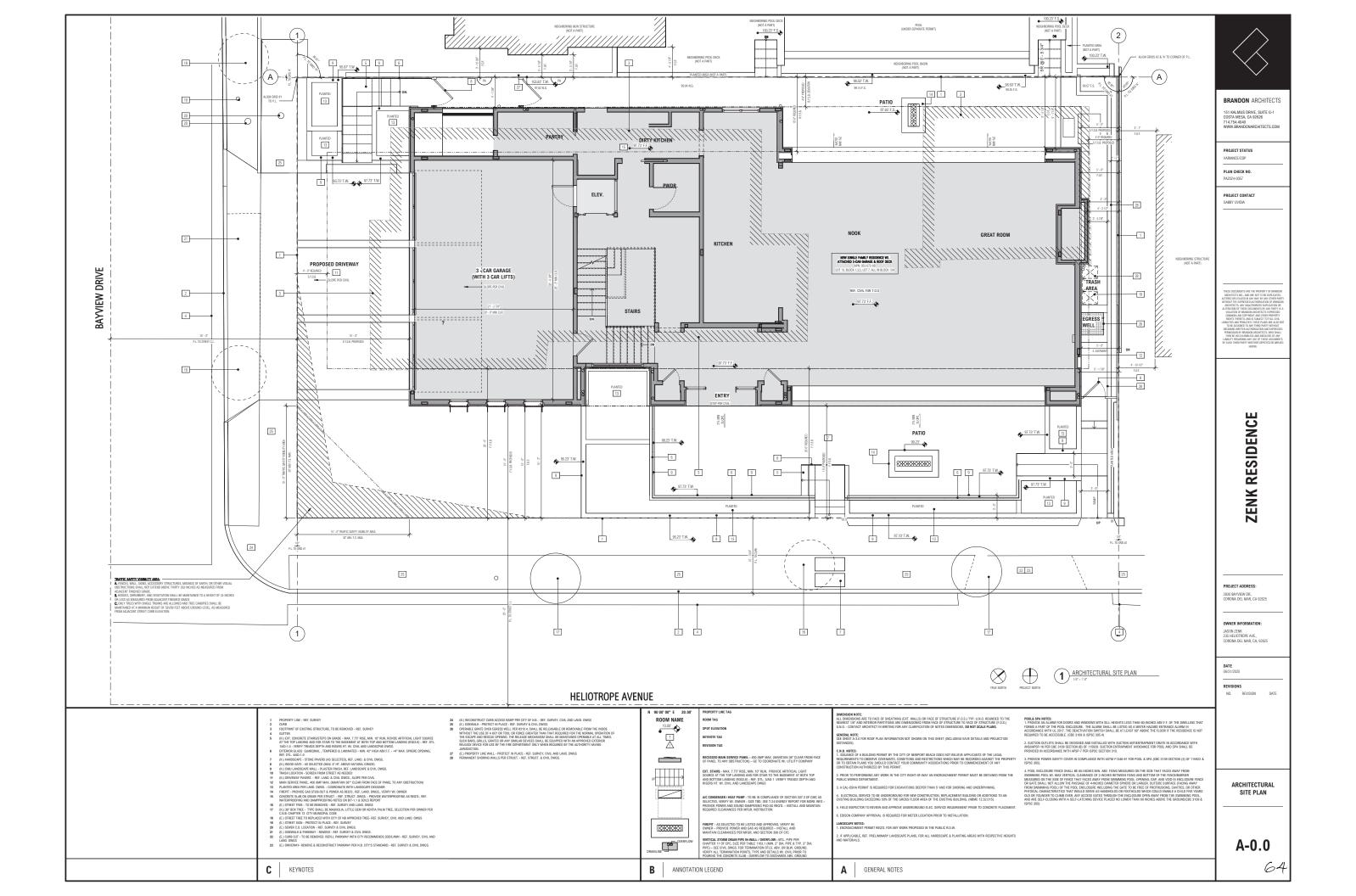
NOTE: SECTION 8770.6 OF THE CALIFORNIA BUSINESS AND PROFESSIONS CODE NOTE: SECTION 8770.6 OF THE CALIFORNIA BUSINESS AND PROFESSIONS CODE STATES THAT THE USE OF THE WORD CERTIFY OR CERTIFICATION BY A LICENSED LAND SURVEYOR IN THE PRACTICE OF LAND SURVEYING OR THE PREPARATION OF MAPS, PLATS, REPORTS, DESCRIPTIONS OR OTHER SURVEYING DOCUMENTS ONLY CONSTITUTES AN EXPRESSION OF PROFESSIONAL OPINION REGARDING THOSE FACTS OR FINDINGS WHICH ARE THE SUBJECT OF THE CERTIFICATION AND DOES NOT CONSTITUTE A WARRANTY OR GUARANTEE, EITHER EXPRESSED ON HIM IF D EITHER EXPRESSED OR IMPLIED.

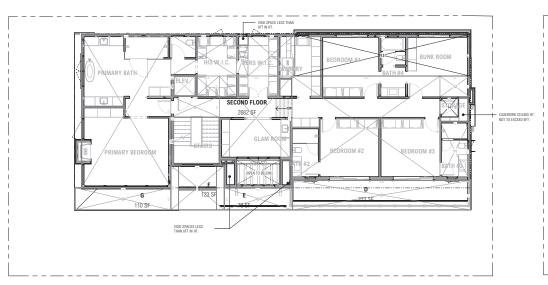
2830 BAYVIEW DRIVE CORONA DEL MAR, CA 92625 (APN: 052-072-08)

**TOPOGRAPHIC MAP** 

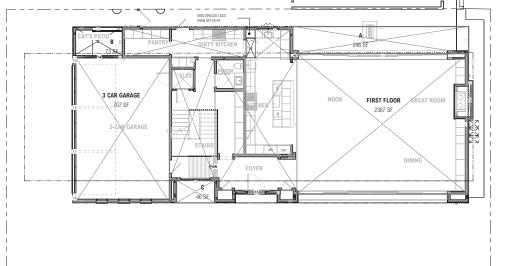
SHEET NO.

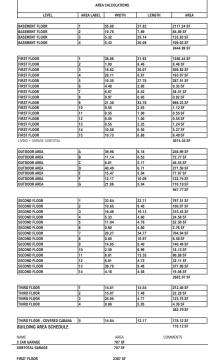
**1** OF





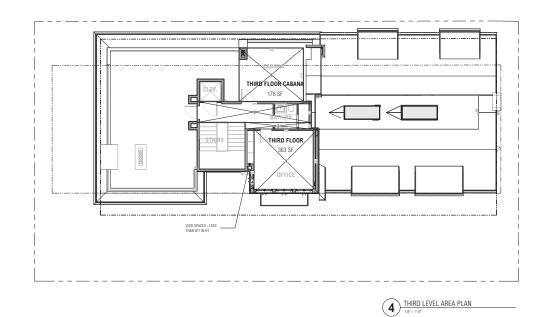
3 SECOND LEVEL AREA PLAN
1/8" = 1"-0"

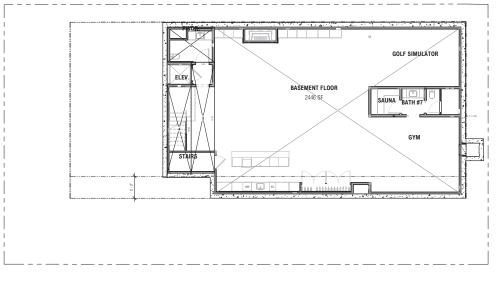




	3	25.95	4.77	123.75 SF		
THIRD FLOOR	4	0.86	5.05	4.36 SF		
				362.79 SF		
THIRD FLOOR - COVERED CABAN	1 5	14.64	12.17	178.12 SF		
BUILDING AREA SCHEDULE		•		178.12 SF		
NAME		AREA		COMMENTS		
3 CAR GARAGE		707 SF				
SUBTOTAL GARAGE		707 SF				
FIRST FLOOR		2367 SF				
SECOND FLOOR		2682 SF				
THIRD FLOOR		363 SF	363 SF			
SUBTOTAL LIVABLE		5413 SF				
GROSS FLOOR AREA = 5,401 + 707 =	6,108 S.F.					
BASEMENT FLOOR		2446 SF				
SUBTOTAL SUBTERRANEAN LIVAL	2446 SF					
THIRD FLOOR CABANA		178 SF				
SURTOTAL THIRD FLOOR CARANA	178 SF					
	GRAND TOTAL					

OPEN AREA		
OF EN AREA		
NAME	AREA	COMMENT
REAR PATIO	246 SF	
REAR PATIO	73 SF	
REAR PATIO	46 SF	
FIRST FLOOR SUB-TOTAL	365 SF	
FRONT BEDROOM BALCONIES	272 SF	
FRONT BEDROOM BALCONIES	78 SF	
FRONT BEDROOM BALCONIES	123 SF	
FRONT BEDROOM BALCONIES	110 SF	
SECOND FLOOR SUB-TOTAL	582 SF	
GRAND TOTAL	947 SF	





BASEMENT LEVEL AREA PLAN

188" = 11-0"

FIRST LEVEL AREA PLAN
1/8" = 1"-0"

ZENK RESIDENCE

**BRANDON** ARCHITECTS

PROJECT STATUS
VARIANCE/CDP

PLAN CHECK NO.
PA2024-0057

PROJECT CONTACT GABBY UVIDIA

PROJECT ADDRESS: 2830 BAYVIEW DR., CORONA DEL MAR, CA 9

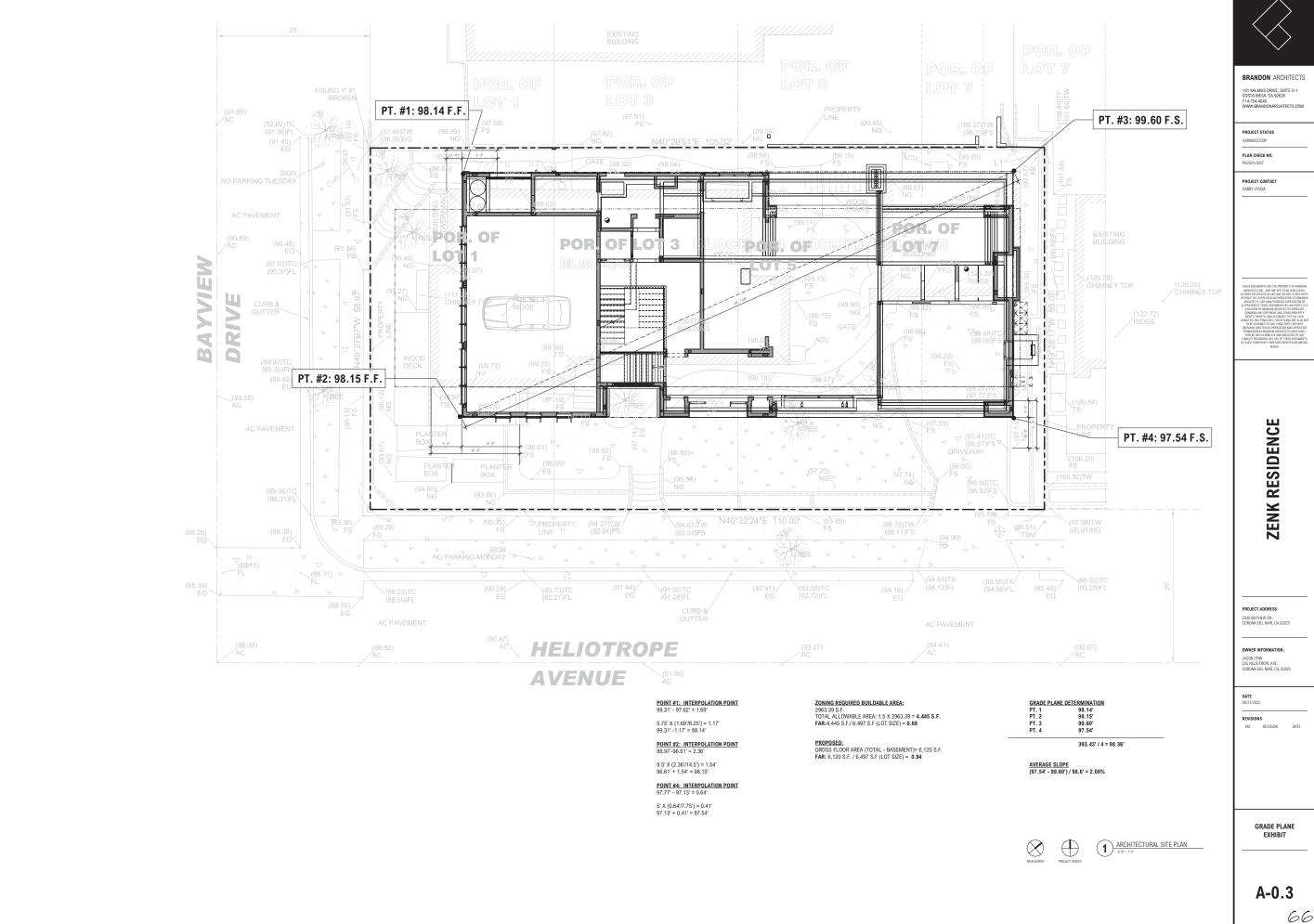
OWNER INFORMATION: JASON ZENK 235 HELIOTROPE AVE., CORONA DEL MAR, CA, 92825

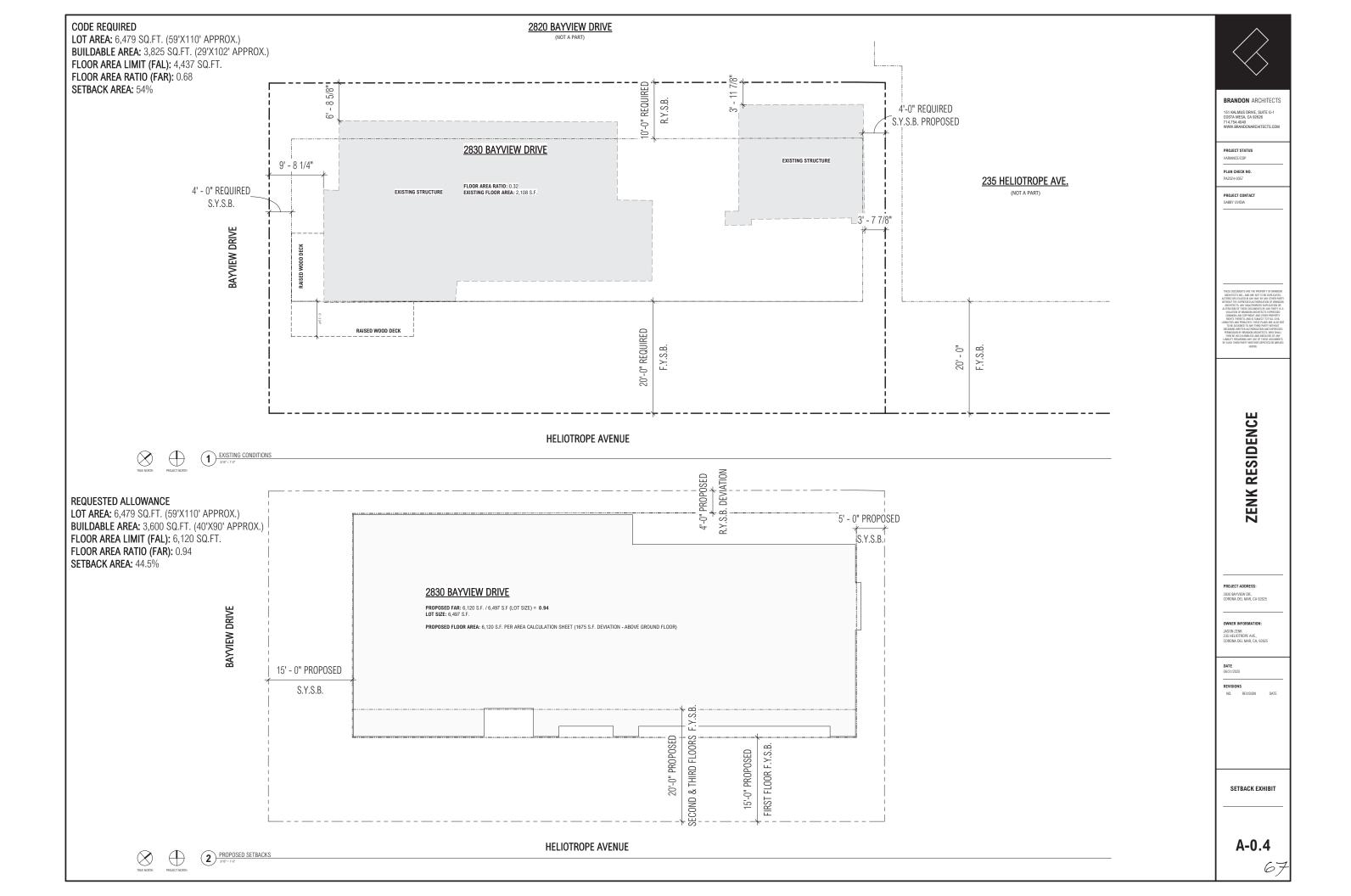
**DATE** 06/31/2025

NO. REVISION

AREA PLANS

A-0.2





# GRADING PLAN

2830 BAYVIFW AVENUE NEWPORT BEACH, CA 92625 APN: 052-072-08

### **GENERAL NOTES**

- ALL WORK SHALL CONFORM TO CHAPTER 15 OF THE NEWPORT BEACH MUNICIPAL CODE (NBMC), THE PROJECT SOILS REPORT AND SPECIAL REQUIREMENTS OF THE PERMI
- 2. DUST SHALL BE CONTROLLED BY WATERING AND/OF JUST PALLIATIVE.
- 3. SANITARY FACILITIES SHALL BE MAINTAINED ON THE SITE DURING CONSTRUCTION PERIOD.
- 4. WORK HOURS ARE LIMITED FROM 7:00 AM TO 6:30 PM MONDAY THROUGH FRIDAY; 8:00 AM TO 6:00 PM SATURDAYS; AND NO WORK ON SUNDAYS AND HOLIDAYS PER SECTION 10-28 OF THE NBMC.
- 5. NOISE, EXCAVATION, DELIVERY AND REMOVAL SHALL BE CONTROLLED PER SECTION 10-28
- 6. THE STAMPED SET OF THE APPROVED PLANS SHALL BE ON THE JOB SITE AT ALL TIMES. 7. PERMITTEE AND CONTRACTOR ARE RESPONSIBLE FOR LOCATING AND PROTECTING
- APPROVED SHORING, DRAINAGE PROVISION AND PROTECTIVE MEASURES MUST BE USED TO PROTECT ADJOINING PROPERTIES DURING THE GRADING OPERATION.
- 9. CESSPOOLS AND SEPTIC TANKS SHALL BE ABANDONED IN COMPLIANCE WITH THE UNIFORM PLUMBING CODE AND APPROVED BY THE BUILDING OFFICIAL.
- 10. HAUL ROUTES FOR IMPORT OR EXPORT OF MATERIALS SHALL BE APPROVED BY THE CITY TRAFFIC ENGINEER AND PROCEDURES SHALL CONFORM WITH CHAPTER 15 OF THE NBMC
- 11. POSITIVE DRAINAGE SHALL BE MAINTAINED AWAY FROM ALL BUILDINGS AND SLOPE
- 12. FAILURE TO REQUEST INSPECTIONS AND/OR HAVE REMOVABLE EROSION CONTROL DEVICES ON-SITE AT THE APPROPRIATE TIMES SHALL RESULT IN A "STOP WORK" ORDER.
- 13. ALL PLASTIC DRAINAGE PIPES SHALL CONSIST OF PVC (SDR 35) WITH GLUED JOINTS.
- 14. NO PAINT, PLASTER, CEMENT, SOIL, MORTAR OR OTHER RESIDUE, SHALL BE ALLOWED TO ENTER STREETS CURBS, GUTTERS OR STORM DRAINS. ALL MATERIALS AND WASTES SHALL BE REMOVED FROM THE SITE

### **GRADING NOTES**

- 1. GRADED SLOPES SHALL BE NO STEEPER THAN 2 HORIZONTAL TO 1 VERTICAL
- 2. FILL SLOPES SHALL BE COMPACTED TO NO LESS THAN 90 PERCENT RELATIVE COMPACTION OUT TO THE FINISHED SURFACE.
- ALL FILLS SHALL BE COMPACTED THROUGHOUT TO THE MINIMUM OF 90 PERCENT RELATIVE COMPACTION AS DETERMINED BY ASTM TEST METHOD 1557, AND APPROVED BY THE SOILS ENGINEER. COMPACTION TEST SHALL BE PERFORMED APPROXIMATELY EVERY TWO FEET IN VERTICAL HEIGHT AND OF SUFFICIENT QUANTITY TO ATTEST TO THE OVERALL COMPACTION EFFORT APPLIED TO THE FILL AREAS.
- 4 AREAS TO RECEIVE FILL SHALL BE CLEARED OF ALL VEGETATION AND DERRIS SCARIFIED AND APPROVED BY THE SOILS ENGINEER PRIOR TO PLACING OF THE FILL.
- 5. FILLS SHALL BE KEYED OR BENCHED INTO COMPETENT MATERIAL.
- 6. ALL EXISTING FILLS SHALL BE APPROVED BY THE SOILS ENGINEER OR REMOVED BEFORE ANY ADDITIONAL FILLS ARE ADDED.
- ANY EXISTING IRRIGATION LINES AND CISTERNS SHALL BE REMOVED OR CRUSHED IN PLACE AND BACKFILLED AND APPROVED BY THE SOILS ENGINEER.
- 8. THE EXACT LOCATION OF THE SUBDRAINS SHALL BE SURVEYED IN THE FIELD FOR LINE
- ALL TRENCH BACKFILLS SHALL BE COMPACTED THROUGHOUT THE MINIMUM OF 90
  PERCENT RELATIVE COMPACTION, AND APPROVED BY THE SOILS ENGINEER. THE
  BUILDING DEPARTMENT MAY REQUIRE CORING OF CONCRETE FLATWORK PLACED OVER
  UNTESTED BACKFILLS TO FACILITATE TESTING.
- 10. THE STOCKPILING OF EXCESS MATERIAL SHALL BE APPROVED BY THE BUILDING
- ALL CUT SLOPES SHALL BE INVESTIGATED BOTH DURING AND AFTER GRADING BY AN ENGINEERING GEOLOGIST TO DETERMINE IF ANY STABILITY PROBLEM EXISTS. SHOULD EXCAVATION DISCLOSE ANY GEOLOGICAL HAZARDS OR POTENTIAL GEOLOGICAL HAZARDS, THE ENGINEERING GEOLOGISTS SHALL RECOMMEND AND SUBMIT NECESSARY TREATMENT TO THE BUILDING DEPARTMENT FOR APPROVAL.
- 12 WHFRE SUPPORT OR BUTTRESSING OF CUT AND NATURAL SLOPE IS DETERMINED TO BE WHERE SUPPORT OR BUT TRESSING OF CUT AND MATURAL SLOPE IS DETERMINED TO BE NECESSARY BY THE ENGINEERING GEOLOGIST AND SOILS ENGINEER, THE SOILS ENGINEER WILL OBTAIN APPROVAL OF DESIGN, LOCATIONS AND CALCULATIONS FROM THE BUILDING DEPARTMENT PRIOR TO CONSTRUCTION.
- 13. THE ENGINEERING GEOLOGIST AND SOILS ENGINEER SHALL INSPECT AND TEST THE CONSTRUCTION OF ALL BUTTRESS FILLS AND ATTEST TO THE STABILITY OF THE SLOPE AND ADJACENT STRUCTURES UPON COMPLETION.
- 14. THE ENGINEERING GEOLOGIST SHALL PERFORM PERIODIC INSPECTIONS DURING
- NOTIFICATION OF NONCOMPLIANCE: IF IN THE COURSE OF FULFILLING THEIR NO III:CATION OF NONCOMPLIANCE: IF IN THE COURSE OF FULFILLING ITHEIR RESPONSIBILITY, THE CIVIL ENGINEER, THE SOULS ENGINEER. THE ENGINEER ITHE SOULS ENGINEER. THE ENGINEER ING GEOLOGIST OR THE TESTING AGENCY FINDS THAT THE WORK IS, NOT BEING DONE IN CONFORMANCE WITH THE APPROVED GRADING PLANS, THE DISCREPANICES SHALL BE REPORTED IMMEDIATELY IN WRITING TO THE PERSON IN CHARGE OF THE GRADING WORK AND TO THE BUILDING INSPECTOR. RECOMMENDATIONS FOR CORPECTIVE MEASURES, IF NECESSARY, SHALL BE SUBMITTED TO THE BUILDING DEPARTMENT FOR ADDROVIAL.

### NOTES TO OWNER, CONTRACTOR, & ARCHITECT

- CONSTRUCTION CONTRACTOR AGREES THAT IN ACCORDANCE WITH GENERALLY CONSTRUCTION CONTRACTOR AGREES THAT IN ACCORDANCE WITH GENERALLY ACCEPTED CONSTRUCTION PRACTICES, CONSTRUCTION CONTRACTOR WILL BE REQUIRED TO ASSUME SOLE AND COMPLETE RESPONSIBILITY FOR JOB SITE CONDITIONS DURING THE COURSE OF CONSTRUCTION OF THE PROJECT, INCLUDING SAFETY OF ALL PERSONS AND PROPERTY; THAT IS REQUIREMENT SHALL BE MADE TO APPLY CONTINUOUSLY AND NOT BE LIMITED TO NORMAL WORKING HOURS.
- NO UTILITY SEARCH WAS CONDUCTED. A UTILITY SEARCH BY THE CONTRACTOR SHALL BE CONDUCTED AND IT SHALL BE THE CONTRACTOR'S RESPONSIBILITY TO TAKE DUE PRECAUTIONARY MEASURES TO PROTECT THE UTILITIES OR STRUCTURES FOUND ON THE SITE AND TO NOTIFY THE OWNERS OF THE UTILITIES IMMEDIATELY UPON THEIR DISCOVERY.
- 3. EARTHWORK AND OTHER CONSTRUCTION ITEM QUANTITIES SHOWN ON THESE PLANS ARE ESTIMATES FOR PERMITTING PURPOSES ONLY AND SHALL NOT USED FOR CONSTRUCTION COST ESTIMATES OR FOR BIDDING PURPOSES. THE CONTRACTOR SHALL DEVELOP OWN QUANTITIES FOR BIDDING PURPOSES.
- A SOILS INVESTIGATION MUST BE MADE BY A QUALIFIED SOILS ENGINEER AND/OR GEOLOGIST. SOIL AND EARTH ACCEPTABILITY ARE NOT UNDER PURVIEW OR THE RESPONSIBILITY OF THE DESIGN ENGINEER FOR THIS PLAN. CIVILSCAPES ENGINEERING DOES NOT TEST OR OBSERVE SOIL CONDITIONS PRIOR TO, DURING OR AFTER CONSTRUCTION AND HAS NO RESPONSIBILITY FOR SOILS (EARTH) STRUCTURES.
- ALL RETAINING WALL DESIGNS ARE TO BE BUILT PER STRUCTURAL ENGINEER'S PLAN AND
- 6. REFER TO SOILS REPORT FOR GRADING RECOMMENDATIONS.
- 7. CONTRACTOR SHALL VERIFY EXISTING ELEVATION, PROTECT ALL EXISTING UTILITIES, AND
- 8. TOPOGRAPHIC SURVEY SHOWN HEREON FOR REFERENCE PURPOSES ONLY.
- 9. TOPOGRAPHIC SURVEY PREPARED BY: APEX LAND SURVEYING, INC., 8512 OXLEY CIRCLE, HUNTINGTON BEACH, CA. PHONE: 714-488-5006.
- VERIFY EXISTING TOPOGRAPHIC ELEVATIONS AND NOTIFY CIVILSCAPES ENGINEERING OF ANY CONFLICTS PRIOR TO CONSTRUCTION.
- 11. NO UTILITY SEARCH WAS CONDUCTED. CONTRACTOR SHALL PROTECT UTILITIES OR STRUCTURES FOUND ON THE SITE AND NOTIFY CIVILSCAPES ENGINEERING OF ANY
- 12. EXTERIOR FOUNDATIONS WALLS SHALL COMPLY WITH THE DETAILS AS SHOWN BELOW: 13. PAD ELEVATION IS ASSUMED TO BE BASED ON ARCHITECTURAL FLOOR PLAN WITH AT
- LEAST 5" THICK CONCRETE AND 4" THICK BASE WITH VAPOR BARRIER PER SOILS REPORT. CONTRACTOR TO VERIFY WITH LATEST APPROVED SOILS REPORT AND STRUCTURAL ENGINEER FOR EXACT SLAB RECOMMENDATIONS 14. A PUBLIC WORKS DEPARTMENT ENCROACHMENT PERMIT INSPECTION IS REQUIRED BEFORE THE BUILDING DEPARTMENT PERMIT FINAL CAN BE ISSUED. AT THE TIME OF PUBLIC WORKS DEPARTMENT INSPECTION, IF AN OF THE EXISTING PUBLIC IMPROVEMENTS SURROUNDING THE SITE IS DAMAGED, NEW CONCRETE SIDEWALK, CURB
- AND GUTTER, AND ALLEY/STREET PAVEMENT WILL BE REQUIRED AND 100% PAID BY THE OWNER. SAID DETERMINATION AND THE EXTENT OF THE REPAIR WORK SHALL BE MADE AT THE DISCRETION OF THE PUBLIC WORKS INSPECTOR.
- . AN APPROVED ENCROACHMENT PERMIT IS REQUIRED FOR ALL WORK ACTIVITIES WITH THE PUBLIC RIGHT OF WAY. AN ENCROACHMENT AGREEMENT IS REQUIRED FOR ALL NON-STANDARD IMPROVEMENTS WITHIN THE PUBLIC RIGHT-OF-WAY. 16. ALL WORK RELATED TO WATER IN THE PUBLIC RIGHT-OF-WAY SHALL BE PERFORMED BY
- A C-34 LICENSED PIPELINE CONTRACTOR OR AN A LICENSED GENERAL ENGINEERING CONTRACTOR.
- PERFORMED BY A C-42 LICENSED SANITATION SEWER CONTRACTOR OR AN A LICENSED GENERAL ENGINEERING CONTRACTOR.
- 18 EXISTING STREET TREE(S) SHALL BE PROTECTED IN PLACE
- 9. REMOVE EXISTING NON-GRASS PARKWAY ELEMENTS BETWEEN SIDEWALK AND CURB. REPLACE WITH SOD TO MATCH EXISTING AS REQUIRED.
- 20. REFER TO SOILS REPORT FOR GRADING RECOMMENDATIONS AND OVER-EXCAVATION REMOVALS AND COMPACTION REQUIREMENTS.
- 21. PIPE MATERIAL MAY BE SUBSTITUTED IF APPROVED BY ENGINEER.
- 22. INCLUDE ALL REQUIRED JOINTS AND FITTINGS. PER MANUFACTURER'S INSTALLATION 23. UTILITIES SHALL BE CONSTRUCTED AND INSTALLED PER CALIFORNIA PLUMBING CODE AND CITY PLUMBING CODE. SERVICE LINES AND METER SIZES SHALL BE CONFIRMED BY PLUMBING ENGINEER OR CONTRACTOR PRIOR TO CONSTRUCTION.
- 24. CONSTRUCT TRENCH, BEDDING, AND BACKFILL PER MANUFACTURER'S INSTALLATION INSTRUCTIONS, ASTM D 2321, AND SOILS REPORT.
- 25. ALL FIXTURES, EQUIPMENT, PIPING AND MATERIALS SHALL BE LISTED.
- 26. CONTRACTOR SHALL VERIFY ELEVATION PRIOR TO CONSTRUCTION AND NOTIFY ENGINEER OF ANY DISCREPANCIES.
- 27. ALL PRIVATE IRRIGATION SPRINKLER HEADS SHALL BE INSTALLED AND POSITIONED IN A MANNER THAT WILL NOT CAUSE IRRIGATION OVERSPRAY ONTO THE PUBLIC

### **EROSION CONTROL NOTES**

- ALL LOOSE SOIL AND DEBRIS SHALL BE REMOVED FROM PAVED SURFACES, AREAS UPON
- SEDIMENT CONTROL MEASURES (I.E. GRAVEL BAGS OR EQUIVALENT) SHALL BE IMPLEMENTED AT THE PERIMETER OF ALL DISTURBED SOIL AREAS TO CONTROL RUN-ON
- 3. GRAVEL BAGS AND NECESSARY MATERIALS SHALL BE AVAILABLE ON SITE AND STOCKPILED AT CONVENIENT LOCATIONS TO FACILITATE RAPID CONSTRUCTION OF TEMPORARY DEVICES OR TO REPAIR ANY DAMAGED EROSION CONTROL MEASURES, WHEN RAIN IS IMMINENT. A STAND-BY CREW SHALL BE MADE AVAILABLE AT ALL TIMES DURING THE
- MATERIALS AND WASTE WITH THE POTENTIAL TO POLLUTE URBAN RUN-OFF SHALL BE USED IN ACCORDANCE WITH LABEL DIRECTIONS AND SHALL BE STORED IN A MANNER THAT EITHER PREVENTS CONTACT WITH RAINFALL OR CONTAINS CONTAMINATED RUN-OFF FOR TREATMENT AND DISPOSAL.

### CC&R'S

ISSUANCE OF A BUILDING PERMIT BY THE CITY OF NEWPORT BEACH DOES NOT RELIEVE APPLICANTS OF THE LEGAL REQUIREMENTS TO OBSERVE COVENANTS, CONDITIONS AND RESTRICTIONS WHICH MAY BE RECORDED AGAINST THE PROPERTY OR TO OBTAIN PLANS YOU SHOULD CONTACT YOUR COMMUNITY ASSOCIATIONS PRIOR TO

### **PUBLIC WORKS NOTES**

- PRIOR TO PERFORMING ANY WORK IN THE CITY RIGHT-OF-WAY AN ENCROACHMENT PERMIT MUST BE OBTAINED FROM THE PUBLIC WORKS DEPARTMENT.
- A PUBLIC WORKS DEPARTMENT ENCROACHMENT PERMIT INSPECTION IS REQUIRED BEFORE THE BUILDING DEPARTMENT PERMIT FINAL CAN BE ISSUED. AT THE TIME OF PUBLIC WORKS DEPARTMENT INSPECTION, IF ANY OF THE EXISTING PUBLIC IMPROVEMENTS SURROUNDING THE SITE IS DAMAGED, NEW CONCRETE SIDEWALK, CURB AND GUITER, AND ALLEY/STREET PAVEMENT WILL BE REQUIRED. ADDITIONALLY, IF EXISTING UTILITIES INFRASTRUCTURE ARE DEEMED SUBSTANDARD, A NEW 1-INCH WATER SERVICE, WATER METER BOX, SEVER LATERAL AND/OR CLEANOUT WITH BOX AND LID WILL BE REQUIRED. 100% OF THE COST SHALL BE BORNE BY THE PROPERTY OWNER (MUNICIPAL CODES 142 420 AND 14 A08.30). SAID DETERMINATION AND THE EXTENT OF THE REPAIR WORK SHALL BE MADE AT THE DISCRETION OF THE PUBLIC WORKS INSPECTOR. CONTRACTOR IS RESPONSIBLE TO MAINTAIN THE PUBLIC RIGHT OF WAY AT ALL TIMES DURING THE CONSTRUCTION PROJECT. A STOP WORK NOTICE MAY BE ISSUED FOR ANY DAMAGE OR UNMAINTAINED PORTION OF THE PUBLIC RIGHT OF WAY. OR UNMAINTAINED PORTION OF THE PUBLIC RIGHT OF WAY.
- AN ENCROACHMENT AGREEMENT IS REQUIRED FOR ALL NON-STANDARD IMPROVEMENTS WITHIN THE PUBLIC RIGHT OF WAY, ALL NON-STANDARE IMPROVEMENTS SHALL COMPLY WITH CITY COUNCIL POLICY L-6.
- 4. CONTRACTOR REMOVE ALL EXISTING DECORATIVE MATERIAL WITHIN THE PUBLIC RIGHT-OF-WAY.
- 5. ALL LANDSCAPING WITHIN THE PUBLIC RIGHT-OF-WAY SHALL HAVE A MAXIMUM GROWTH CHARACTERISTIC OF 36-INCHES.
- 6. ALL PRIVATE IRRIGATION SPRINKLER HEADS SHALL BE INSTALLED AND POSITIONED IN A MONNER THAT WILL NOT CAUSE IRRIGATION OVERSPRAY ONTO THE PUBLIC

# OWNER

HADDAD FAMILY 128 KINGS PL. NEWPORT BEACH, CA 92663

### **ARCHITECT** BRANDON ARCHITECTS

COSTA MESA, CA 92626 714.754.4040

# **CIVIL ENGINEER**

WILL ROLPH CIVILSCAPES ENGINEERING, INC 28052 CAMINO CAPISTRANO, STE 213 LAGUNA NIGUEL. CA WILL @CIVIL SCAPES COM

# **SURVEYOR**

APEX LAND SURVEYING, INC. VILLA PARK, CA 92861 714.488.5006 APEXLSINC@GMAIL.COM

### **SOILS ENGINEER**

REFER TO GEOTECHNICAL INVESTIGATION FOR ADDITIONAL INFORMATION

EGS CONSULTANTS, INC 375-C MONTE VISTA AVENUE COSTA MESA, CA 92627

# NOTE:

MONUMENT PROPERTY CORNERS OR OFFSETS BEFORE GRADING.





# SHEET INDEX

- C1 TITLE SHEET
  C2 GRADING PLAN
  C3 STORM DRAIN PLAN
  C4 CONSTRUCTION POLLUTION PREVENTION PLAN
- C5 GEOTECHNICAL NOTES
  C6 GEOTECHNICAL NOTES (CONTD.)

# SITE DATA:

SITE ADDRESS: 2830 BAYVIEW AVE., NEWPORT BEACH APN: 052-072-08
GENERAL PLAN LAND USE: RS-D SINGLE UNIT RESIDENTIAL DETACHED ZONING DISTRICT: R-1 SINGLE-UNIT RESDENTIAL

### **EARTHWORK QUANTITIES**

SHRINKAGE 5%±

### GEOTECHNICAL CERTIFICATION

THIS GRADING PLAN HAS BEEN REVIEWED BY THE UNDERSIGNED AND FOUND TO BE IN CONFORMANCE WITH THE RECOMMENDATIONS AS OUTLINED IN THE FOLLOWING SOILS

ENTITLED: GEOTECHNICAL INVESTIGATION

FEBRUARY 26, 2024

FIRM NAME: EGA CONSULTANTS, INC

NO. REVISION DATE

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28052 CAMINO CAPISTRANO, LAGUNA NIGUEL, CA 92677 949.464.8115 info@civilscapes

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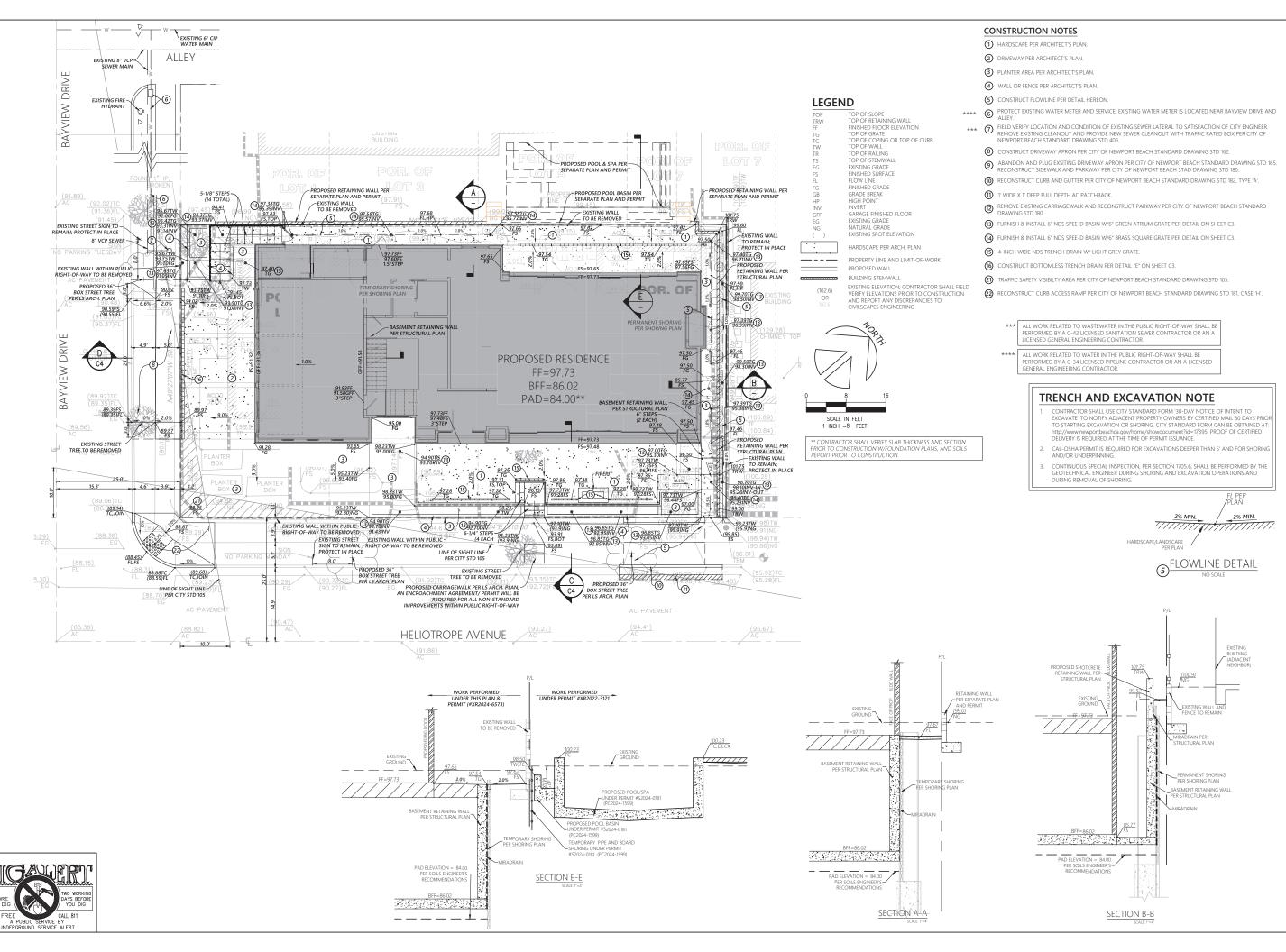
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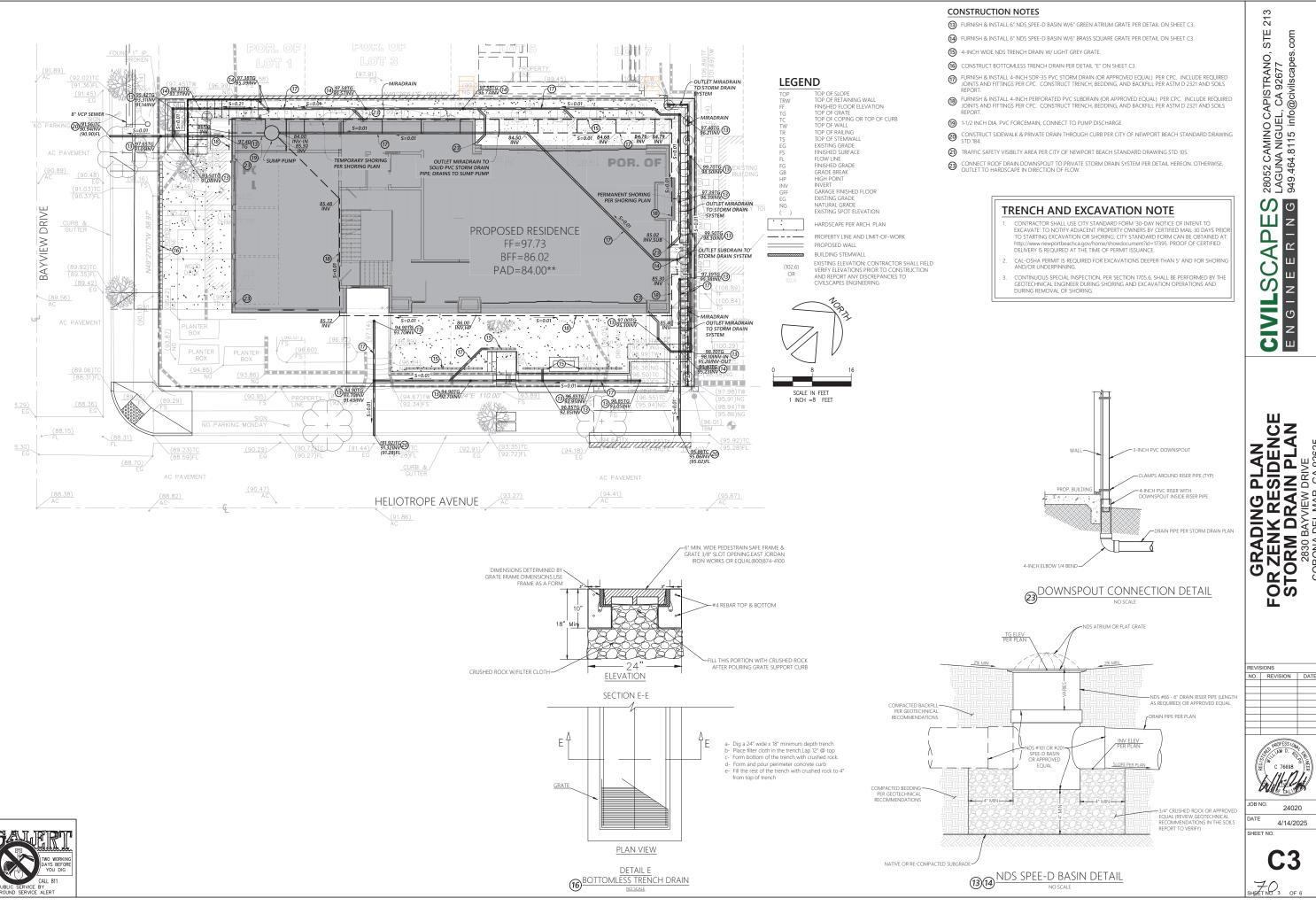


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CIVILSCAPES ENGINEERING

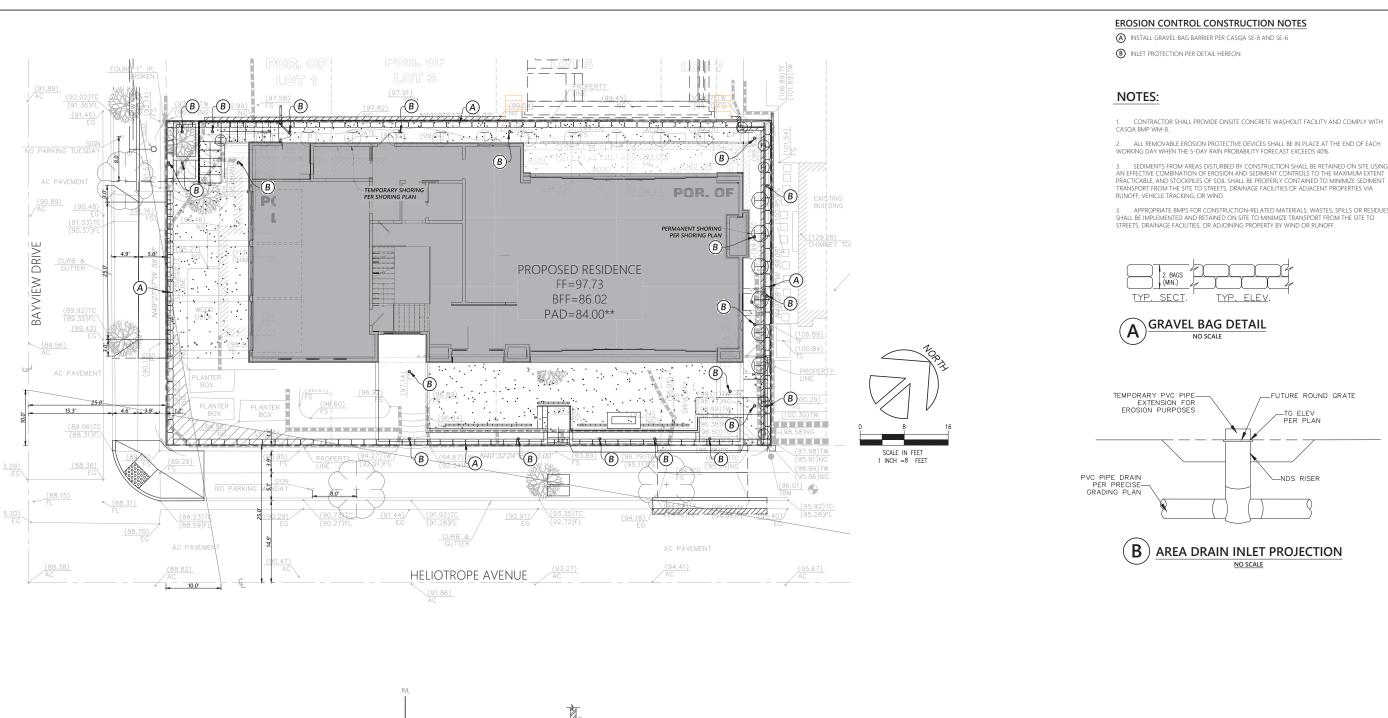
# GRADING PLAN FOR ZENK RESIDENCE GRADING PLAN

NO. REVISION DATE



NO. REVISION DATE





BASEMENT RETAINING WA PER STRUCTURAL PLAN

SECTION C-C



NO. REVISION DATE 4/14/2025

213

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PLAN

SECTION D-D

### FINDINGS

### Subsurface Soils

As encountered in our test borings, the site is underlain by, fill and native materials as follows:

Fill soils were encountered in each of the borings to a depth of approximately 2 to 21/2 feet b.g. The fill soils consist generally yellowish own, moists, fine- to medium-grained, silty sand with gravel.

Based on the laboratory results, the project on-site soil maximum density shall be 121.0 pcf with an optimum moisture content of 10.0%.

Underlying the fill materials are older Quaternary-age terrace/paralic deposits as encountered in each of the test borings (B-1 through B-3) to the maximum depths explored (15 ft b.g.). The native soils consist generally of light to medium brown, damp to moist, medium dense to dense, fine- to medium-grained sand and silty sand. The native materials become more dense and less weathered with depth.

Based on the laboratory results and the findings of our geotechnical investigation, the on site soils consist of non-expansive silty sand and sand. No additional measures for mitigation of highly expansive or highly plastic soils are warranted.

Based on our geotechnical study of the site, our review of available reports and literature and our experience, it is our opinion that the proposed improvements at the site are feasible from a geotechnical standpoint. There appear to be no significant geotechnical constraints on-site that cannot be mitigated by proper planning, design, and utilization of sound construction practices. The engineering properties of the soil and native materials, and the surface drainage offer favorable conditions for site re-

### development.

### RECOMMENDATIONS

The following sections discuss the principle geotechnical concerns which should be considered for proper site re-development (includes ground level and basement level slabs). The precise limits (elevations) of the basement were not available at this date.

Grading and earthwork should be performed in accordance with the following recommendations and the General Earthwork and Grading Guidelines included in Appendix C. It is our understanding that the majority of grading will be limited to the re-grading of the building pad for the proposed new construction. In general, it is anticipated that the removal and re-compaction of the upper 3 feet within the upper building footprint (slab-on-grade portion) will be required.

Where feasible, we recommend a 5 feet wide envelope be excavated beyond the building limits. The removals should ensure that all old fill and backfill created as part of the previous site use and demolition operations are removed.

Care should be taken to protect the adjacent property improvements. A minimum one foot thick fill blanket should be placed throughout the exterior improvements (approaches, parking and planter areas). The fill blanket will be achieved by re-working (scarifying) the upper 12 inches of the existing grade.

### Site Preparation

Prior to earthwork or construction operations, the site should be cleared of surface structures and buried obstructions and stripped of any vegetation, trees, and roots in the areas proposed for development. Removed vegetation and debris should then be disposed of off-site. A minimum of 3 feet of the soils below the existing street grade (upper pad) will require removal and recompaction in the areas to receive building pad fill.

Following removals, each excavated area should be inspected by the soils engineer or his designated representative prior to the placement of any fill. Holes or pockets of undocumented fill resulting from removal of buried obstructions discovered during this inspection should be filled with suitable compacted fill.

### Earthwork - Lower Basement Pad

For the basement subgrade, a minimum one foot of compacted fil should be placed. The fill blanket will be achieved by re-working (scarifying) the upper 12 inches of the basement grade. For a more detailed description of the basement

slab section, see "Basement Underslab" section below.

The on-site soils are suitable for reuse as compacted fill, provided the soil is free of organic materials, debris, and rock materials larger than four (4) inches in diameter. After removal of any loose, compressible soils, all areas to receive fill and/or other surface improvements should be scarified to a minimum depth of 12 inches, brought to at least 2 percent over optimum moisture conditions and compacted to at least 90 percent relative compaction (based on ASTM: D 1557). If necessary, import soils for near-surface fills should be predominately granular, possess a very low expansion potential, and be approved by the geotechnical

Lift thicknesses will be dependent on the size and type of equipment used. In general, fill should be placed in uniform lifts not exceeding 8 inches.
Placement and compaction of fill should be in accordance with local grading ordinances under the observation and testing of the geotechnical consultant. We recommend that fill soils be placed at moisture contents at least 2 percent over optimum (based on ASTM: D 1557).

### **Backfill Suitability**

The on-site soils may be used as trench backfill provided they are screened of rock sizes over 4 inches in mean diameter and any and organic matter. Trench backfill should be compacted in uniform lifts (not exceeding 8 inches in compacted thickness) by mechanical means to at least 90 percent relative compaction (ASTM: D1557).

### Geotechnical Design Parameters

The following geotechnical recommendations may used in the design of the proposed

### Conventional Foundation Design

Structures on properly compacted fill may be supported by conventional, continuous or isolated pad footings. Footings should be a minimum of 24 inches deep by 15 inches wide. At this depth footings founded in fill materials may be designed for an allowable bearing value of 1,750 psf and 2,250 psf (for deadplus-live load) for continuous wall and isolated pad footings, respectively. These values may be increased by one-third for loads of short duration, including wind

Continuous perimeter and interior footings should have a minimum width of 15 inches and be reinforced with No. 5 rebar (two at the top and two at the bottom).

Reinforcement requirements may be increased if recommended by the project structural engineer. In no case should they be decreased from the previous

### Lateral Load Resistance - New Building

Footings founded in fill materials may be designed for a passive lateral bearing pressure of 250 pounds per square foot per foot of depth. A coefficient of friction against sliding between concrete and soil of 0.30 may be assumed.

For the upper 12 feet, we recommend a soil unit weight of 120 pcf. The unit weight may be reduced to 110 pcf for the native soils.

### Slabs-on-grade - Conventional Option

Concrete slabs cast against properly compacted fill materials, or approved native material, shall be a minimum of 5 inches thick (actual) and reinforced with No. 4 rebar at 18 inches on center in both directions. The slabs shall be doweled into the footings using No. 4 bars at 24 inches on center. The reinforcement shall be supported on which is the party and the slabs of the sla supported on chairs to insure positioning of the reinforcement at mid-center in

All living area floor mat slabs shall comply with the "Capillary Break" section below, herein (does not apply to basement slab where certified waterproofing

Some slab cracking due to shrinkage should be anticipated. The potential for the slab cracking may be reduced by careful control of water/cement ratios. The contractor should take appropriate curing precautions during the pouring of concrete in hot weather to minimize cracking of slabs. We recommend that a slipsheet (or equivalent) be utilized if crack-sensitive flooring is planned directly on concrete slabs. All slabs should be designed in accordance with structural

### Capillary Break - for Conventional Floor Slab Design

In accordance with the 2022 California Green Building Standards Code Section 4.505.2.1, we provide the following building specification for the subject site (new

New concrete building slabs shall be underlain by 2 inches of clean sand, underlain by a minimum 15 mil-thick moisture barrier (e.g. "Stego Wrap"), with all laps sealed, underlain by 4 inches of ½-inch or larger clean gravel.

We do not advise placing sand directly on the gravel layer as this would reverse the effects of vapor capture and increase soil settlement (due to siltation of fines).

The above specification meets or exceeds the Section 4.505.2.1 requirement.

Mat Foundations:

A mat slab foundation system is recommended for the basement. Mat slabs founded in compacted fill or competent native materials may be designed for an allowable bearing value of 1,500 psf (for dead-plus-live load). These values may be increased by one-third for loads of short duration, including wind or seismic forces. The actual design of the mat slab should be designed by the structural

MIN. DESIGN ITEM RECOMMENDATIONS

allowable bearing pressure: 1,500 psf 250 psf per foot passive lateral resistence:

min. 12 inches with thickened edges (+ mat slab thickness:

no. 5 bars @ 12" o.c. each way, top and

coefficient of friction  $k_s = 90 \text{lbs/in}^3$ Modulus of Subgrade Reaction:

Joints in walls and floors, and between the wall and floor, and penetrations of the wall and floor shall be made watertight using suitable methods and materials (e.g. bentonite "water stops").

### Basement Underslab

We recommend the bottom of the basement excavation be scarified to 12 inches and re-compacted to a minimum 90% relative compaction. The basement pad (12 inches below the bottom of the basement slab) should be overlain by 4 inches of ½-inch gravel covered by filter fabric. To counter against the effects of migrating nuisance/perched water we recommend a minimum 4-inch thick weighted slab (a.k.a. "rat" slab) be poured above the gravel and fabric layer. The waste slab shall be overlain by waterproofing (e.g. "Carlisle Waterproofing Products") which extends up the wall faces, and then overlain by a minimum 4inch "protection slab" and then overlain by a minimum 12-inch structural mat

We recommend the waterproofing be inspected and certified by a trained expert. The protection slab is crucial in preserving the underlying waterproofing from puncture or damage during construction. Steel reinforcement is not required for the protection slab or the waste slab.

We recommend a minimum 4,000 psi concrete pour for the mat slab, to be designed by the project structural engineer.

### Waterproofing

Basement wall and slabs shall be waterproofed in accordance with section 1805 of the 2022 CBC. Permanent waterproofing of the basement slab and basement walls is required. Waterproofing shall consist of rubberized asphalt, butyl rubber, or other approved materials capable of bridging modified asphalt, butly fluorer, of other approved ritarials expected in the mon-structural cracks (e.g. "Carlisle Waterproofing Products"). Joint in the membrane shall be lapped and sealed in an approved manner. Protection board shall be used to protect the membrane at the face of basement walls, during and after backfilling. Joints and protrusions in walls and floors, and between the wall and floor, and penetrations of the wall and floor shall be made watertight using suitable methods and materials (e.g. bentonite "Water Stops").

The contractors shall strictly follow the manufacturer's recommendations for the for surface preparation and use of water-proofing products. A third-party waterproofing expert shall be retained to inspect and verify the waterproofing installation.

# Cement Type for Concrete in Contact with On-Site Earth Materials

Concrete mix design should be based on sulfate testing with Section 1904.2 of the 2022 CBC. Preliminary laboratory testing indicates the site soils possess negligible sulfate exposure (13 ppm by volume). Test Results are presented in

# ACI 318-14 BUILDING CODE (Table 19.3.1.1) REQUIREMENTS FOR CONCRETE EXPOSED TO SULFATE-CONTAINING SOLUTIONS

Sulfate Exposure	Water soluble sulfate (SO <sub>4</sub> ) in soil percent by weight	Sulfate (SO <sub>4</sub> ) in water, ppm	Cement Type	Maximum water- cementitious material ratio, by weight, normal weight concrete	Minimum fc <sup>1</sup> , normal-weight and light weight concrete, psi
Negligible [S0]	0.00 ≤ SO <sub>4</sub> < 0.10	0 ≤ SO <sub>4</sub> <150	*****	-	;
Moderate [S1]	0.10 < SO <sub>4</sub> < 0.20	150 < SO <sub>4</sub> < 1500	II,IP(MS), IS(MS),P(MS) I(PM)(MS), I(SM)(MS)	0.50	4000
Severe [S2]	0.20 ≤ SO <sub>4</sub> < 2.00 1500 < SO <sub>4</sub> < V 0.45		4500		
Very Severe [S3]	SO <sub>4</sub> > 2.00	SO <sub>4</sub> > 10,000	V plus pozzalan	0.45	4500

As a conservative approach, cement with a maximum water/cement ratio of 0.50 and a concrete strength fc of 3,000 psi should be used for concrete in contact with on-site earth materials (min. 4,000 psi applies to the basement slab).

### Retaining/Basement Wall Design

To date, the precise limits and heights of the proposed basement and exterior retaining walls are not known. However, based on conceptual plans, we assume shoring will be required along the north, east, and west side yards.

All retaining/basement and landscape wall footings will be embedded into fill compacted to a minimum 90% relative compaction or competent native materials. The following equivalent fluid pressures may be used in the design of the site retaining walls assuming free draining conditions (clean sand or gravel

	Equivalent Fluid Pressure (EFP				
Condition	Level	2:1 Slope			
Active Pressures	45 pcf	85 pcf			
At-Rest Pressures	65 pcf	120 pcf			
Coefficient of Friction	0.30				
Passive Pressures	250 psf per	foot			
Saismic Pressures	19 pcf				

The above passive pressure values do not contain an appreciable factor of safety. Therefore, the structural engineer should apply the applicable factors of

This office shall be contacted to provide additional recommendations if actual conditions are different than those assumed above.

### Lateral Pressure - New Retaining Walls

A passive earth pressure of 300 pounds per square foot per foot of depth, to a maximum value of 3,500 pounds per square foot, may be used to determine lateral bearing resistance for footings. An increase of one-third of the above values may also be used when designing for short duration wind and seismic

The above lateral resistance values are based on footing placed directly against competent native soils or compacted fill. In cases where footing sides are formed, all backfill placed against the footings should be compacted to at least 90 percent of the applicable maximum dry density value.

In accordance with Section 1803.5.12 of the 2022 CBC, for design purposes, a seismic earth pressure of 19 pcf (additional equivalent fluid press used for the shoring and the basement wall design. For this, the allowable soil

GRADING PLAN
FOR ZENK RESIDENCE
GEOTECHNICAL NOTES
2830 BAYVIEW DRIVE
CORONA DEL MAR, CA 92625

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28052 CAMINO CAPISTRANO, LAGUNA NIGUEL, CA 92677 949.464.8115 info@civilscapes

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Based on our subsurface investigation (including the consolidation test results), we estimate that the total settlement for the proposed structure will be on the order of ¾-inch. Differential settlement is expected to be less than 0.36 inch over 30 feet. These settlement values are expected to be within tolerable limits for properly designed and constructed foundations.

### Retaining Wall Backfill Material

It is recommended that a minimum 2-foot thick layer of free-draining granular material (less than 5 % passing the No. 200 sieve) be placed against the back face of the retaining walls. This material should be approved by the geotechnical engineer. This layer of granular material should be separated from the adjacent soils using a suitable geotextile fabric. If the layer of free-draining material is not covered by an impermeable surface, such as a structure or pavement, a 12-inch thick layer of a low permeability soil should be placed over the backfill to reduce surface water migration to the underlying soils.

All retaining wall backfill should be placed and compacted under engineering controlled conditions in the necessary layer thickness to ensure a minimum in place density of 90 percent of the maximum dry density as determined by the Modified Proctor test (ASTM D1557). Care should be taken to avoid overcompaction of the soils behind the retaining walls, and the use of heavy compaction equipment should be avoided.

The retaining walls shall be provided with water- or damp-proofing in accordance with the architects recommendations. Back drains and chimney drains shall be installed to collect and divert migrating groundwater. As a minimum, each wall shall be drained by placing a 4-inch diameter pipe perforated (faced down) PVC Schedule 40 pipe or approved equivalent, located behind the base of the wall. The pipe shall be covered by %-inch crushed rock at a rate of not less than 2 cubic ft. per linear ft. of pipe surrounded in turn by geofabric such as Supac 4NP

All wall backfill shall be compacted to a minimum 90 percent relative compaction in accordance with ASTM D-1557. Wall back drains shall outlet separately and

This office shall be contacted to provide additional recommendations if actual conditions are different than those assumed above. During construction, drainage devices shall be inspected by a representative of EGA Consultants.

### Landscape/Retaining Wall Waterproofing

In accordance with Section 1805.3 of the 2022 CBC, the retaining walls shall be sealed and waterproofed using the miradrain and miraclay (i.e. Grace 3000) waterproofing system, or equivalent. Joints in the membrane shall be lapped and sealed in an approved manner. Protection board shall be used to protect the membrane during and after backfilling.

The contractors shall strictly follow the manufacturer's recommendations for the for use of water-proofing products.

### Retaining Wall Back Drains Along P.L. if any

Water in the retained earth will be drained into the channel drains at each bay via the miradrain panels which will be attached to the lagging (sealed with miraday waterproofing). Due to space constraints along the side yard property lines, the typical gravel encased backdrains pipes should be replaced with the implementation of the miradrain, waterproofing, and outlet drains at each bay (a.k.a. "J-Drains").

### SHORING PARAMETERS

Note: The current residence pad is roughly 90 to 100 ft. above MSL; and the proposed finished floor elevation for the basement is currently undetermined. The precise limits of retaining wall construction are not yet available. However, based on preliminary drawings by Brandon Architects, it appears that shoring will be required where temporary construction slopes cannot be created or are feasible such as along the rear

In cases where the shoring may need to double as landscape walls, and/or landscape features (e.g. exterior stairs, boundary fences, walls), then those sections should be designed as permanent. The structural engineers should interface with the architect and landscape designer to determine location of exterior

The limits of the shoring soldier piles and temporary excavation lay back cuts shall be shown in the future grading plan.

The following equivalent fluid pressures may be used in the design of the permanent or temporary shoring:

The caissons shall be a minium 24 inches in diameter, embedded a minimum 5 feet into bedrock. Caissons may be designated for both end bearing and friction Caissons may be designed for an allowable bearing capacity of 5,000 psf and a

Proposed Zenk Residential Development 2830 Bayview Drive, Corona Del Mar, CA - Preliminary Soils Report Project No. BA475.1 February 26, 2024

skin friction of 250 psf (neglect the upper 3 feet of fill). The precise caisson dimensions and steel reinforcement shall be determined by the project structural

The minimum clear spacing between the H-beams should be three effective Hbeam diameters, sidewall to sidewall. The maximum clear spacing between H-beams should not exceed five effective H-beam diameters, sidewall to sidewall.

No additional significant surcharge will be added since the structures shall be supported by deepened footings embedded into competent bedrock. The fill/topsoil shall be removed in those areas. All excavation bottoms should be inspected/approved by the geotechnical consultant.

To protect adjacent properties we recommend that the annulus spaces behind the lagging be backfilled with a minium 2-sack slurry. The slurry backfill shall be performed as soon as possible during the shoring installation and shall be monitored/documented by the geotechnical consultant.

After the shoring system is in-place, the excavation of the basement may begin If concrete and slurry is used for backfill, these materials should be allowed to cure prior to excavation of the basement. Care should be taken to ensure that the lagging drops down as the excavation advances. Any gaps in the lagging could cause undermining of the adjacent structures. To prevent caving of the sidewalls, the lagging elements should be forced down either behind the soldier piles or at an appropriate place within the flanges of the "H" and through the existing soils. The slurry materials that were placed within the soldier pile borings may be broken an removed during the lagging process. The lagging ents should not be driven or vibrated into place due to the possible damage that could occur to nearby structures.

These shall be considered minimum requirements and incorporated into the Foundation, Grading, Retaining Wall, and Shoring Plans. The survey monitoring requirements shall be posted in the Shoring Plan by others. This office should review the plans when available.

For cantilever shoring systems, an active earth pressure (equivalent fluid pressure) of 45 pounds per cubic foot may be considered for the on-site fill and the native soils. It should be noted that under this condition, the movement of shoring H-beams are not restrained so that the soil internal strength can be fully

### At-Rest Earth Pressure

If the piles are restrained at the top, then an at-rest earth pressure of 65 pounds per cubic foot should be used in design.

### Passive Pressure

A passive earth pressure increasing of 300 psf per foot of width of a shoring H-beam, per foot of depth, to a maximum value of 3,500 pounds per square foot may be used to determine lateral resistance for the piles (600 psf-ft may be used for temporary shoring). All soldier piles should extend at least 10 feet into competent bedrock. The passive resistance should be ignored for the upper 2 feet of the H-beams embedded below the lowest cut grade

This office shall be contacted to provide additional recommendations if actual conditions are different than those assumed above. During construction, all waterproofing and drainage devices shall be inspected by a representative of EGA Consultants.

### Soldier Pile Installation Observations

All soldier pile drilling and installation should be observed by the project geotechnical consultant to verify that they are cast against the anticipated geotechnical conditions, that pile excavations are properly prepared, that proper dimensions are achieved, and that proper installation procedures are followed.

### Soldier Pile Monitoring by Others

We recommend a minimum of 8 (eight) monitoring points installed by a Licensed Surveying company. At least four of the monitoring points shall be established near each of the side yard property lines on the drilled shoring piles. The settlement monitoring points shall be monitored for horizontal and vertical movement prior and subsequent to the completion of construction, and on a daily basis during the grading and basement construction.

### Stability of Temporary Excavations

During the construction and remedial grading within areas that may not require shoring and recessed portions of the proposed basement, temporary excavations with sidewalls varying up to approximately 12 feet in height will be created. For some portions of the basement excavation, shoring may not required as there may be sufficient space for temporary sloping back cuts. The shoring layout shall be determined when the construction plans are finalized and

Based on the results of our subsurface investigation, it is expected that the temporary excavation sidewalls will expose dense to very dense terrace and bedrock deposits with favorable shear strength.

Vertical cuts will not exceed 5 feet in height. Based upon the nature of the subsurface soils, vertical cuts may extend to 4 feet, and laid back (tapered) at an inclination of 1:1 (horizontal to vertical). The temporary excavation parameters

# shall be shown in the future grading plan.

Temporary unsupported sidewalls constructed at the recommended maximum gradient are expected to remain stable for the duration of the remedial grading operations; however, all temporary slopes should be observed by a representative of the project geotechnical consultant for any evidence of

All applicable requirements of the California Construction and General Industry Safety Orders, the Occupational Safety and Health Act of 1970, and the Construction Safety Act should also be followed.

A grade beam, reinforced continuously with the garage footings, should be constructed across the garage entrance, tying together the ends of the garage footings. This grade beam should be embedded at the same depth as the adjacent perimeter footings.

Exterior slabs shall conform to the requirements for interior slabs except that the moisture barrier may be omitted and the thickness may be reduced to 4-inches, and the reinforcement may be reduced to No. 3 rebar at 24 inches on center.

Control joints should be provided at a maximum spacing of 8 feet on center in two directions for slabs and at 6 feet on center for sidewalks. Control joints are intended to direct cracking. Minor cracking of exterior concrete slabs on grade

Expansion or felt joints should be used at the interface of exterior slabs on grade and any fixed structures to permit relative movement.

New fences or garden wall footings, if any, should be founded a minimum of 18-inches into approved firm materials. To reduce the potential for unsightly cracks due to expansion forces, we recommend inclusion of construction joints at 8-ft to

### Surface Drainage

Surface drainage shall be controlled at all times. Positive surface drainage should be provided to direct surface water away from structures and toward the street or suitable drainage facilities. Ponding of water should be avoided adjacent to the structures. Roof gutter discharge should be directed away from the building areas through solid PVC pipes to suitable discharge points. Area

Proposed Zenk Residential Development 2830 Bayview Drive, Corona Del Mar, CA - Preliminary Soils Report Project No. BA475.1 February 26, 2024

rains should be provided for planter areas and drainage shall be directed away

Small slopes, when occurring, shall be properly drained and landscaped (such as non-homogeneous, drought-tolerant, deep-rooted plants) and maintained.

In general, the more the surface water can be controlled, the less surface water will infiltrate the underlying earth materials. All drainage should be controlled and diverted away from the on site structures and to a suitable discharge at Bayview Drive or Heliotrope Avenue.

### Review of Plans

The specifications and parameters outlined in this report shall be considered minimum requirements and incorporated into the Grading, Foundation, Shoring, Landscape, Shoring plans and Pool/Spa (if any). This office should review the Plans when available. If approved, the geotechnical consultant shall sign/stamp the applicable Plans from a geotechnical standpoint.

Based on the findings of our geotechnical investigation and our professional experience working on similar sites in the area, the proposed construction will not impact the stability/safety of the subject or surrounding sites. Geologic hazards such as landsliding, settlement, or slippage are not present or future factors at the subject or

Geotechnical observations/testing should be performed during all grading operations, including excavations, waterproofing, drain device installments, removals, filling, compaction, and backfilling, etc.

### GEOTECHNICAL OBSERVATION AND TESTING DURING CONSTRUCTION

We recommend that a qualified geotechnical consultant be retained to provide geotechnical engineering services, including geotechnical observation/lesting, during the construction phase of the project. This is to verify the compliance with the design, specifications and or recommendations, and to allow design changes in the event that subsurface conditions differ from those anticipated.

Geotechnical observations/testing should be performed at the following stages:

- During ANY grading operations, including excavation, removal, filling,
- compaction, and backfilling, etc.

  After excavations for footings/grade beams and/or drilling for soldier
  piles/caissons, if any to verify the adequacy of underlying materials.

  After excavation for retaining wall footings to verify the adequacy of underlying
- earth materials.
- earth internals.

  During/after installation of water proofing for retaining walls, if any prior to installation of sub-drain/backfilling.

  During/after installation of retaining wall sub-drain, prior to backfilling.

  During compaction of retaining wall backfill materials to verify proper

After pre-soaking of new slab sub-grade earth materials, prior to pouring

- Verification of the placement of the slab underlayment prior to pouring concrete. Prior to slab pours to ensure proper subgrade compaction, capillary breaks, and moisture barriers.
- Placement of waterproofing at cold joints and penetrations (e.g. bentonitic "Water Stops").
- During backfill of drainage and utility line trenches, to verify proper compaction. When/if any unusual geotechnical conditions are encountered.

Please schedule an inspection with the geotechnical consultant prior to the pouring of ALL interior and exterior slabs (includes waste and protection slabs).

ical services described herein have been conducted in a manner consistent with the level of The geotechnical services described herein have been conducted in a manner consistent with the level of care and skill ordinarily exercised by members of the geotechnical engineering profession practicing contemporaneously under similar conditions in the subject locality. Under no circumstance is any warranty, expressed or implicing, made in connection with the providing of services described herein. Data, interpretations, and recommendations presented herein are based solely on information available to this office at the time work was performed. EGA Consultants will not be responsible for other parties' interpretations or use of the information developed in this report.

The interpolated subsurface conditions should be checked in the field during construction by a representative of EGA Consultants. We recommend that all foundation excavations and grading operations be observed by a representative of this firm to ensure that construction is performed in accordance with the specifications outlined in this report.

We do not direct the contractor's operations, and we cannot be responsible for the safety of others. The contractor should notify the owner if he considers any of the recommended actions presented herein to be

GRADING PLAN
FOR ZENK RESIDENCE
GEOTECHNICAL NOTES (CONTD.
2830 BAYVIEW DRIVE
CORONA DEL MAR, CA 92625 G NO. REVISION DATE

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28052 CAMINO CAPISTRANO, LAGUNA NIGUEL, CA 92677 949.464.8115 info@civilscapes

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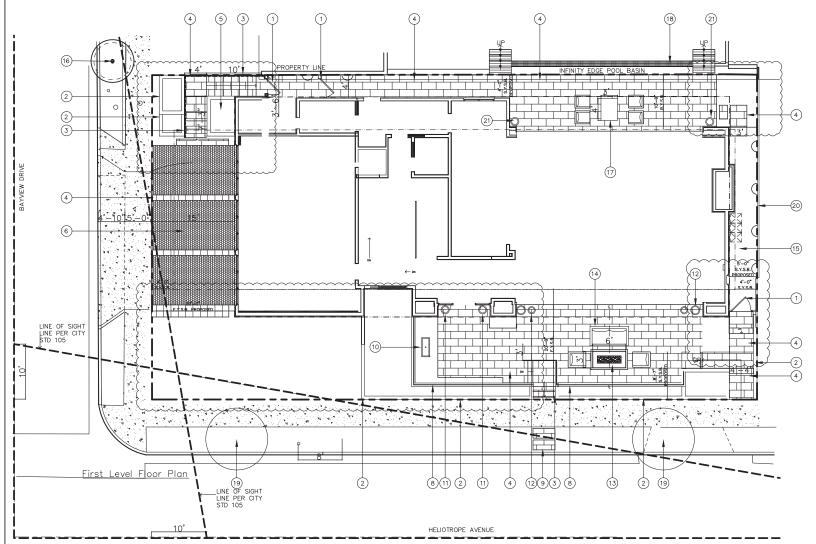
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### Construction Notes:

- 1. Contractor shall assess the proposed property noting the existing conditions and
- Contractor to verify all grades, elevations and measurements in the field prior to construction, including but not limited to: number of steps, riser heights, retaining wall locations and retaining wall heights. Any discrepancies are to be reviewed with 13. Contractor to specify/provide irrigation conduit sleeves, low voltage and electrical the designer.
- Contractor shall stake all proposed work for approval by the client and designer. Contractor to obtain signature approval from the client on the final set of plans and agrees to install the project per the plans. Any field verification discrepancies shall be communicated to the designer prior to construction.
- 4. Contractor shall provide samples of all materials noted on the plans and obtain approval from the client prior to purchase.
- 5. Contractor to protect all existing utilities, hardscape, structures and plant material/trees to remain. Any work done next to existing plant material/trees is to 18. Contractor to confirm placement of all concrete score lines with the client prior to be done by hand to avoid damage to the roots.
- Contractor to obtain a DIG ALERT identification number prior to excavating.
- All construction shall follow the State, County and Local Codes, as well as the CAL OSHA safety orders.
- Contractor must comply with all applicable codes and obtain required permits. Homeowners Association's approval is required if applicable.
- 9. Contractor agrees to follow B.M.P. (Best Management Practices) when installing the proposed plan.
- 10. Contractor assumes responsibility for any damage to underground utilities and will repair or replace at the contractor's expense

- 11. Contractor to confirm that the client has confirmed any potential easements with their Title Company prior to construction. Any easement information is to be communicated to the designer prior to construction.
- proposed work as indicated on the plans, including, not limited to the topography. 12. Contractor to assess the proposed property for proper drainage, verify the locations of new surface drains in concrete prior to sub-base pour, and report any issues to the client and designer.
  - prior to the sub-base pour. All pots shall be supplied with drip irrigation, direct drainage and the option for low voltage lighting.
  - 14. Contractor to determine the optimal soil preparation for the project site in relation to the climate, site conditions and native soil.
  - $15. \quad \text{All work with other trades will be coordinated and managed by the contractor.} \\$
  - 16. Installation of concrete driveways shall follow all applicable Standard Codes.
  - 17. Contractor to follow Applicable Codes for the installation of gas lines electrical, drainage and irrigation.
  - installation, including but not limited to those on driveways, patios, pour in place caps and pool coping. All score lines to be saw cut.
  - All concrete edges shall have a 1/8" trowel edge unless otherwise noted.
  - All stucco edges shall have a 1/8" trowel edge unless otherwise noted.
  - Grout spacing and color to be reviewed with the client and designer prior to install. Contractor to provide a sample of any proposed cement slurries or over-grouts to the client and designer prior to install.
  - 23. All Structural Engineering to be Completed by the Contractor for Items Including but not Limited to: Grading, Retaining Walls, Pools/Spas, Water Features, Patio Cover Structures, etc.



### Setbacks:

- Front: 20'
- (Heliotrope) Rear: 10'
- Left: 4' (Bayview) Right: 4'

# Traffic Safety Visibility Area:

- Fences, Walls, Signs, Accessory Structures, Mounds of Earth, or Other Visual Obstructions shall not Extend above 30 Inches in Height as Measured from Adjacent Finished Grade.
- Hedges, Shrubbery, and Vegetation shall be Maintained to a Height of 24 Inches or Less as Measured from Adjacent Finished Grade.
- Only Trees with Single Trunks are Allowed and Tree Canopies shall be Maintained at a Minimum Height of Seven Feet above Ground Level, as Measured from Adjacent Street Curb Elevation.

# HARDSCAPE LEGEND - FIRST LEVEL FLOOR PLAN

- 6' Tall Non-Climbable Pool Safe Gate, See Sheet L.5, Pool Safe Gate Elevation
- CMU Block Walls (Reference Civil Drawings for Finished Heights), See Sheet L.5, CMU Block Wall Elevation
- Black Iron Handrail (TBD)
- 3.a. Handrail to Conform to Newport Beach City Codes
- 4. Limestone (Antique Finish) with pattern & material to match the ADU backyard at 2820 Bayview:
- Contractor to Confirm Stone (Including Size and Finish) Prior to Purchase
- Stairs: 2" Thick Coordinating Limestone Treads with a 1" Overhang and Coordinating Limestone for the Risers
- Grout Joints to be 1/8" Wide/Grout Color (TBD)
- Contractor to Confirm Sealant Options with the Manufacturer/Client and Seal all Sides
- Dedicated Vegetable Garden Area:
- Contractor to Build Coordinating 18" Tall CMU Block Wall on the Right Side to Contain the Garden. Wall Shall Match Other Block Walls on the Property.
- New Driveway with Cobblestone to match the garage entrance at 2820 Bayview in pattern & material:
- Contractor to Confirm Sealant Options with the Manufacturer/Client and Seal all Sides
- Contractor to Confirm French Drain Location
- 42" Tall Min. (Above Finished Floor) Exterior Frameless Glass
- Guardrail, See Sheet L.5, Glass Wall Elevation: Glass Wall Under Deferred Submittal
- NOT USED
- Parkway Landing with Coordinating Limestone to Match the Property:
- Separate Encroachment Agreement and Encroachment Permit to be Pulled by the Installing Contractor
- 18" Tall Custom Water Feature, 18"Hx18"WX42"L, See Sheet L.5, Water Feature Elevation
- 11. 18" Round Pot:
- Plant: (1x) 15 gal. Olea europaea 'Little Ollie' and (1x) Prostanthera ovafolia 'Variegated' in Each Pot
- Pots to have Drip Irrigation, Drainage and the Option for Low Voltage Lighting
- Contractor to Confirm Pot Locations Prior to Sub-Base Pour with the Client
- Finish with 3/8" Del Rio Gravel

- 12. 20" Round Pot and 16" Round Pot:
- 12.a. 20" Round Pot: (1x) 5 gal. Feijoa sellowiana
- 16" Round Pot: (1x) 5 gal. Leucadendron salignum 'Winter Red', (3x) 1 gal. Aeonium 'Salad Bowl'
- Pots to have Drip Irrigation, Drainage and the Option for Low Voltage Lighting
- Contractor to Confirm Pot Locations Prior to Sub-Base Pour with the Client
- Finish with 3/8" Del Rio Gravel
- 13. 3' Wide x 6' Long x 18" Tall Gas Fire Pit, See Sheet L.5, Frontyard Gas Fire Pit Elevation. Stone veneer to match the ADU backyard vertical stone at 2820 Bayview.
- 14. 7' Long Sofa and (2x) Deep Seat Chairs
- 15. Tiger Turf Pet Turf Dog Run:
- Contractor to Provide a Dedicated Irrigation Valve, Irrigation System and Hose Bib (Confirm Hose Bib Location)
- Contractor to Confirm Turf Variety and Install Per Manufacturer's Guidelines
- 16. Existing City of Newport Beach street tree, replace with City of Newport Beach approved tree, see Planting Sheet for specification
- 17. Coffee table and (4x) deep seat chairs
- 18. Neighboring Property, Refer to 2820 Bayview Plans for
- Specifications
- New City of Newport Beach Street Tree Location, See Planting Sheet for Specification
- 20. Existing Property Fence to be Protected in Place
- 21. 18" Round Pot:
- 21.a. Plant: (1x) 15 gal. Camellia japonica 'Kumasaka'
- Pots to have Drip Irrigation, Drainage and the Option for Low Voltage Lighting
- Contractor to Confirm Pot Locations Prior to Sub-Base Pour with the Client
- Finish with 3/8" Del Rio Gravel



ADDRESS: 120 TUSTIN AVE. SUITE #227, NEWPORT BEACH, CA 92663

CONTRACTOR TO FIELD VERIFY ALL MEASUREMENTS AND CONFIRM DETAILS TO PROVIDE ACCURATE BID

DATE 04.17.2025

02.16.2024 03.05.2024 03.08.2024 03.25.2024 06.28.2024 07.01.2024 09.04.2024 11.06.2024

02.07.2025

DRAWN BY: CVZ SHEET

ZENK RESIDENCE 2830 BAYVIEW DR CORONA DEL MAR, CA 92625

CONCEPTUAL LANDSCAPE PLAN

# Plant List:

1	Tant List.									
	TREE SCHEDULE	SIZE	QTY	HEIGHTxWIDTH	WUCOLS RATING		HEDGE SCHEDULE	SIZE	QTY	HEIGHTxWIDTH
	Camellia japonica 'Kumasaka' Kumasaka Camellia (In Pots)	15 gal.	2	8' x 6'	Moderate		Buxus sempvirens 'Suffruticosa' Dwarf English Boxwood (Spaced 2' On Center)	5 gal.	52	3' x 4'
$\bigcirc$	Citrus x meyeri Dwarf 'Multi' Improved Meyer Lemon Tree	24" box	1	9' x 9'	Moderate		Ligustrum japonicum 'Texanum' Staked Wax Leaf Privet	15 gal.	2	8' x 6' (Maintained 6' x 2')
	Feijoa sellowiana	15 gal.	2	10'-15' x 10'-15'	Low		SHRUB SCHEDULE	SIZE	QTY	HEIGHTxWIDTH
	Pineapple Guava (In Pots)					(AE)	Aeonium urbicum 'Salad Bowl' Salad Bowl Aeonium	1 gal.	23	3' x 3'
(F)	Feijoa sellowiana 'Multi' Pineapple Guava	15 gal.	1	10'-15' x 10'-15'	Low		Leucadendron salignum 'Winter Red' Winter Red Cone Bush	5 gal.	2	4' x 4'
(X)	Feijoa sellowiana 'Multi' Pineapple Guava	24" box	1	10'-15' x 10'-15' (Maintained 10' x 6')	Low		(In Pots)			
KD.				( ,		(LO)	Lomandra longifolia 'Katrinus Deluxe' Katrinus Deluxe Mat Rush	5 gal.	51	3' x 4'
	Howea forsteriana Kentia Palm (Must be a Minimum 5' Brown Trunk Height. See Additional 'City Tree Requirements' Below	36" box	2	5-40' x 3-10'	Moderate	C	Olea europaea 'Little Ollie' Dwarf Olive Shrub (In Pots)	15 gal.	2	6' x 6' (Maintained 3' x 3')
$\bigcirc$	Mangifera indica 'Dwarf Variety' Mango Tree Julie	24" box	1	15' x 15'	Moderate	ML	Westringia fruticosa 'Moming Light' Low Coast Rosemary	5 gal.	13	4' x 4'
$\odot$	Maytenus boaria 'Single Trunk' Mayten Tree (See 'City Tree Requirements' Below)	36" box	1	30' x 20'	Moderate		Prostanthera ovafolia "Variegata" Variegated Australian Mint (In Pots)	15 gal.	2	6' x 5'
	Olea europaea 'Multi' Field Grown Olive (Contractor to Provide Photos/Specs)	14' Diameter	4	30' x 30' (Maintained 15' x 15')	Low	(LE)	Leucadendron salignum 'Winter Red' Winter Red Cone Bush	5 gal.	16	4' x 4'
$\odot$	Plumeria rubra 'White' White Plumeria	24" box	1	20' x 20' (Maintained 8' x 8')	Low	SA	Salvia leucantha ' Santa Barbara' Mexican Sage Bush	5 gal.	19	3' x 3'
						•	Lomandra confertifolia ssp. pallida 'Pom Pom' Shorty Mat Rush	1 gal.	90	2' x 3'

### GROUNDCOVER SCHEDULE SIZE HEIGHTxWIDTH WUCOLS RATING 6" x 24" Sedum rupestre Angelina Sedum Dichondra argentea 6" x Cascading Moderate Silver Falls (In Pots) Erigeron glaucus 'Cape Sebastian' 4" x 24" Cape Sebastian Seaside Daisy (In Pots) Erigeron glaucus 'Cape Sebastian' 4" x 24" Cape Sebastian Seaside Daisy (In Pots) Agrostis pallens 12" x Spreading West Coast Native Grass (Purchased as Sod from West Coast Turf) Viola hederacea Mud Flats 8" x 6" SHRUB SCHEDULE SIZE HEIGHTxWIDTH WUCOLS RATING Rosa x 'Cecile Brunner' Pink 'Staked' 10' x Climbing Moderate Cecile Brunner Climbing Rose 20' x Climbing Trachelospermum jasminoides 'Madison' Staked 15 gal. Moderate Madison Star Jasmine (Wired to the Wall) (Contractor to Confirm Wiring Layout)

### Planting Notes:

- 1. All plant material shall meet industry standards, and the client and designer have the right to refuse any inadequate plant material.
- Stake trees and hedges according to industry standards.
- Specimen trees and artificial turf to be planted in accordance with the purchasing companies installation guidelines. Contractor to determine the 'face' of any specimen trees with the client or
- Planter beds to be finished with EB Stone Microbark, Aguinaga 'Deco Bark' or Aguinaga 'Forest Floor'. Contractor to review/confirm the options with the Client prior to purchase.
- Contractor assumes responsibility for all finish grades and surface drainage. Any issues shall be communicated to the client or designer with a proposed solution.
- The following plant quantities are a computerized count of all species. Contractor shall verify the plant quantities and notify the landscape designer of any discrepancies between the number of plants shown on the plan and the totals listed below.
- Plants (on the Front Door Level) have been Reviewed in Accordance with the ASPCA Toxic Plant List for Dogs. Client to Report any Issues with the Proposed Plants to the Contractor/Designer Prior to Purchase (Client is Aware that Ligustrum japonicum 'Texanum' can be Toxic and Consented to Using it.)

### Irrigation Notes:

- 1. Landscape Contractor shall coordinate the location of all irrigation timers with the General
- 2. Contractor shall install an automatic Netafim drip irrigation system and low precipitation heads where applicable.
- Contractor shall Zone the Valves.
- Contractor to provide a dedicated valve for landscape pots.
- Artificial turf to be supplied with low precipitation heads and drainage.

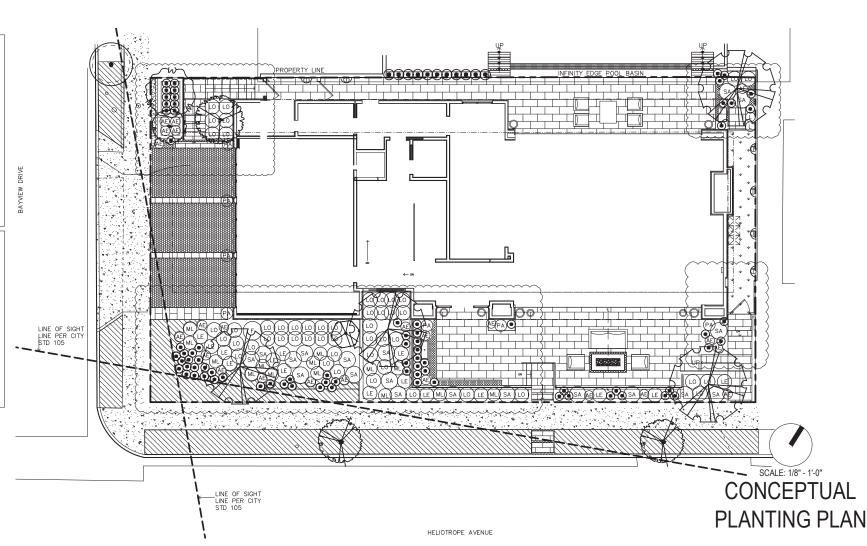
ZENK RESIDENCE 2830 BAYVIEW DR CORONA DEL MAR, CA 92625

# Traffic Safety Visibility Area:

- Fences, Walls, Signs, Accessory Structures, Mounds of Earth, or Other Visual Obstructions shall not Extend above 30 Inches in Height as Measured from Adjacent Finished
- Hedges, Shrubbery, and Vegetation shall be Maintained to a Height of 24 Inches or Less as Measured from Adjacent Finished Grade.
- Only Trees with Single Trunks are Allowed and Tree Canopies shall be Maintained at a Minimum Height of Seven Feet above Ground Level, as Measured from Adjacent Street Curb Elevation.

### City Tree Requirements:

- Prior to Taking Delivery of the Tree, the Tree must be Submitted to the City Arborist for Approval (or the Landscape Manager or Tree Section Crew Chief).
- The Replacement Tree must be Planted in the City's Right-of-Way along the Heliotrope Ave Frontage of 2830 Bayview Dr.
- Per City Ordinance, Irrigation must be Supplied to the Newly Planted Trees, Regardless of Installation of Artificial Turf.
- Separate Encroachment Agreement must be Pulled.



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Moderate

Moderate

WUCOLS RATING

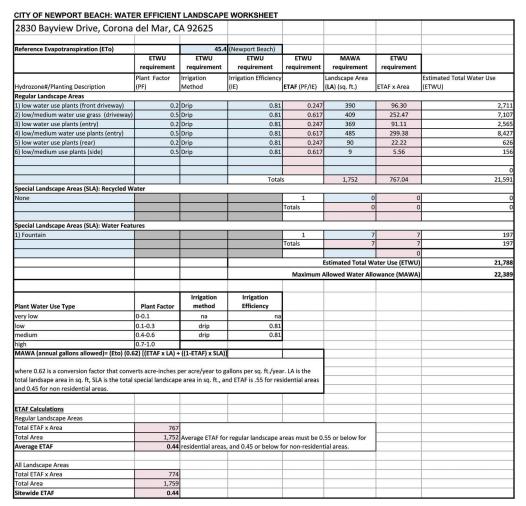
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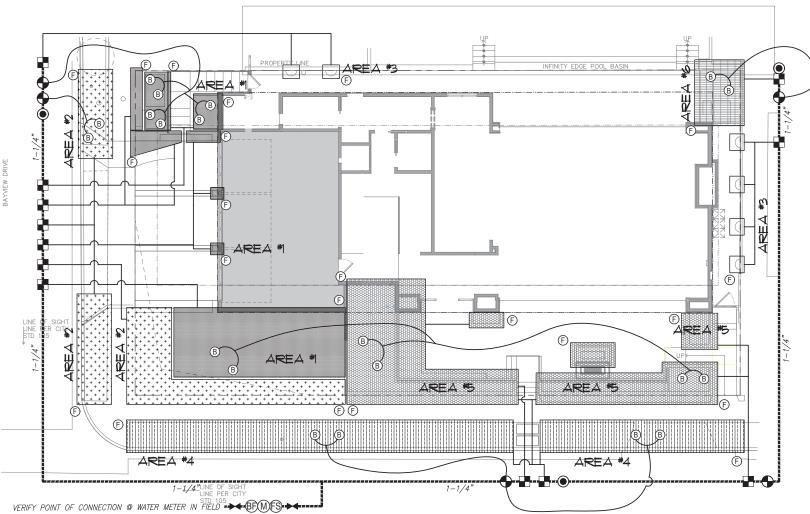


ADDRESS: 120 TUSTIN AVE, SUITE C #227, NEWPORT BEACH, CA 92663

CONTRACTOR TO FIELD VERIFY ALL MEASUREMENTS AND CONFIRM DETAILS TO PROVIDE ACCURATE BID

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IRRIGATION LEGEND

SYMBOL	DESCRIPTION	PSI	GPM	AREA	MANUFACTURER / MODEL		
1	NOT USED	30	-	-	RAINBIRD MPR NOZZLES		
2	NOT USED	30	-	-	RAINBIRD MPR NOZZLES		
3	NOT USED	30	-	-	RAINBIRD MPR NOZZLES		
®	NOT USED	30	-	-	NOT USED		
①	TREE BUBBLER	30	0.50	-	RAINBIRD RWS-B-C-1402-36"LONG MODEL		
B	NOT USED	30	-		NOT USED		
Ē	MANUAL FLUSH VALVE				NETAFIM COMPACT BALL VALVE ON FLEX LINE IN ROUND BOX		
•	REMOTE CONTROL VALVE				RAINBIRD 100-DVF IN BOX, SIZE AS SHOWN		
	REMOTE DRIP ZONE CONTROL VALVES, INSTALL VALVES WITH WY	RAINBIRD XCZ-100-FLOW IN BOX, SIZE AS SHOWN					
M	GATE VALVE, LINE SIZE		SPEARS COMPACT 2000 PVC VALVE IN BOX, LINE SIZE OR EQUAL				
•	QUICK COUPLER VALVE IN GROUND BOX	RAINBIRD 44-RC					
M	MASTER CONTROL VALVE, NORMALLY OPEN VALVE IN BOX	SUPERIOR 3100, 1" DIA					
€3	FLOW SENSOR	CST-FSI-B10-001					
<b>®</b> ₽	FEBCO 825 Y BACKFLOW DEVICE LOCATED IN STAINLESS STEEL C	AGE			BRONZE FIXTURE ON BRASS WORK UP		
	AUTOMATIC IRRIGATION CONTROLLER, SS WALL MOUNT CABINET	SE	HUNTER PRO-C CONTROLLER WITH WIRELESS SOLAR SYNC USE ADD ON MODULES AS NECESSARY PER STATION COUNT				
	DRIP LINE				NETAFIM. 3/4" POLY CV TECHLINE, TLCV4-12, SPACE @ 12" OC		
	LATERAL (NON-PRESSURE) PIPE, <2" SCH 40 PVC, =>2" CL315, SIZE	AS S	HOWN		12" BELOW GRADE		
	MAIN LINE (PRESSURE) PIPE, <3" SCH 40 PVC, =>3" CL315, SIZE AS	SHOV	/N		24" BELOW GRADE		
	IRRIGATION SLEEVE, SCH 40, SIZE AS SHOWN				24" BELOW PAVED SURFACES		
	WIRE SLEEVE, SCH 40, SIZE AS SHOWN	24" BELOW PAVED SURFACES					

# NOTE(S)

- 1. CONTRACTOR SHALL PROVIDE A SOIL FERTILITY REPORT AFTER FOUNDATION HAS BEEN POURRED AND THE SITE HAS BEEN ROUGH GRADED. CONTRACTOR SHALL AMEND THE SOILS BASED ON THE SOIL FERTILITY REPORT RECOMMENDATIONS.
- CONTRACTOR SHALL APPLY MYCORRHIZAE IN THE FORM OF: TRI-C MYCO PACKS IN PLANTING PITS AND/OR TRI-C MYCO DRENCH OVER TURF AND GROUND COVER AREAS PER MFG. RECOMMENDATIONS
- PER THE WATER EFFICIENT LANDSCAPE ORDINANCE NBMC CODE CHAPTER 14 THE SOILS
  MANAGEMENT REPORT SHALL BE PROVIDED BY CONTRACTOR AS A DEFERRED SUBMITTAL AND
  BASED ON THE RECOMMENDATIONS OUTLINED ON THE SOIL FERTILITY REPORT AND NBMC CODE AS IT APPLIES.
- 5. THE OWNER SHALL PROVIDE A 'CERTIFICATION OF COMPLETION PACKAGE' PER THE CITY OF NEWPORT BEACH LANDSCAPE DESIGN STANDARDS UPON COMPLETING THE LANDSCAPE CONSTRUCTION AS PER APPROVED PLANS.
- THE SYMBOLS AND LINES HEREON ARE SHOWN DIAGRAMMATICALLY FOR GRAPHIC CLARITY AND ALL IRRIGATION EQUIPMENT SHALL BE INSTALLED IN PLANTERS WITH LATERALS LINES AT 12" DEPTH AND MAINLINE AT A 18" DEPTH, SLEEVING SHALL BE 24" BELOW PAVED SURFACES.

# PROJECT INFORMATION

DATE: 04.16.2025 JASON ZENK 2830 BAYVIEW DRIVE OWNER: NEWPORT BEACH, CA 92625

PROJECT LOCATION: 2830 BAYVIEW DRIVE NEWPORT BEACH, CA 92625 TOTAL LANDSCAPE AREA: 1,752 SF (PLANTINGS) +7 SF SLA (FOUNTIAN) TOTAL LANDSCAPE AREA = 1,759 SF DOMESTIC POTABLE WATER SUPPLY SHALL BE USED FOR IRRIGATION WATER IN THIS LANDSCAPE

# DRIP IRRIGATION NOTE(S)

- ALL LANDSCAPED AREAS AS SHOWN SHALL USE UNDERGROUND DRIP LINE PER LEGEND. NO OVERHEAD SPRAY IN DESIGN.
- 2) PROVIDE 2" OF TOPSOIL COVER OVER ALL DRIP LINES, SET PVC MANIFOLDS WITH 6" OF SOIL COVER OVER TOP OF PIPE.
- 3) PROVIDE [2] UNDERGROUND TREE BUBBLERS AT EACH TREE PER LEGEND





ADDRESS: 120 TUSTIN AVE. SUITE C #227, NEWPORT BEACH, CA 92663

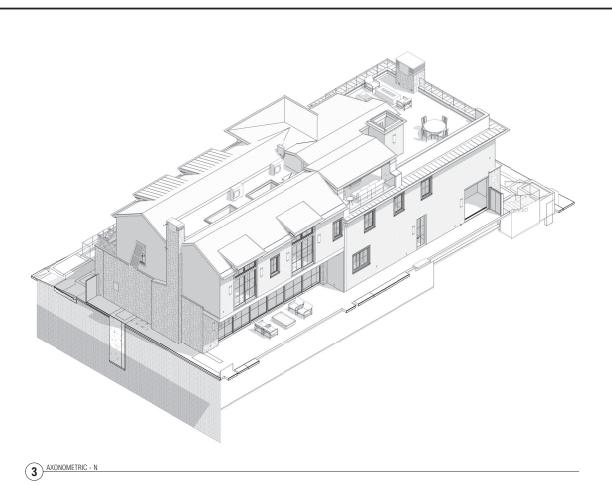
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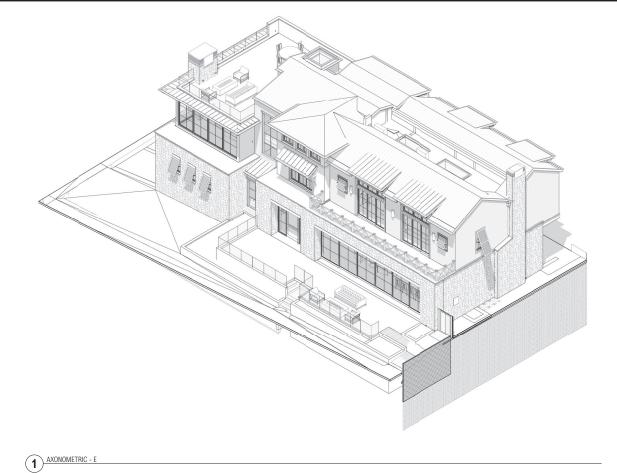
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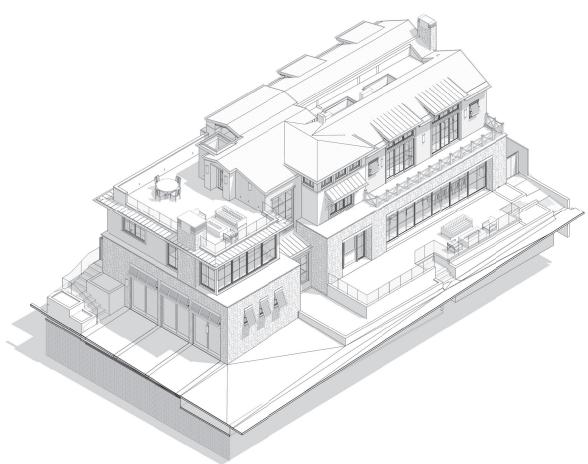
PROJECT NAME: ZENK RESIDENCE

ZENK RESIDENCE 2830 BAYVIEW DR, CORONA DEL MAR, CA 92625

IRRIGATION PLAN







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ZENK RESIDENCE

PROJECT STATUS
VARIANCE/CDP

PLAN CHECK NO.
PA2024-0057

PROJECT CONTACT GABBY UVIDIA

PROJECT ADDRESS: 2830 BAYVIEW DR.

OWNER INFORMATION:

JASON ZENK
236 HEI NOTRIDE AVE

DATE 06/02/2025

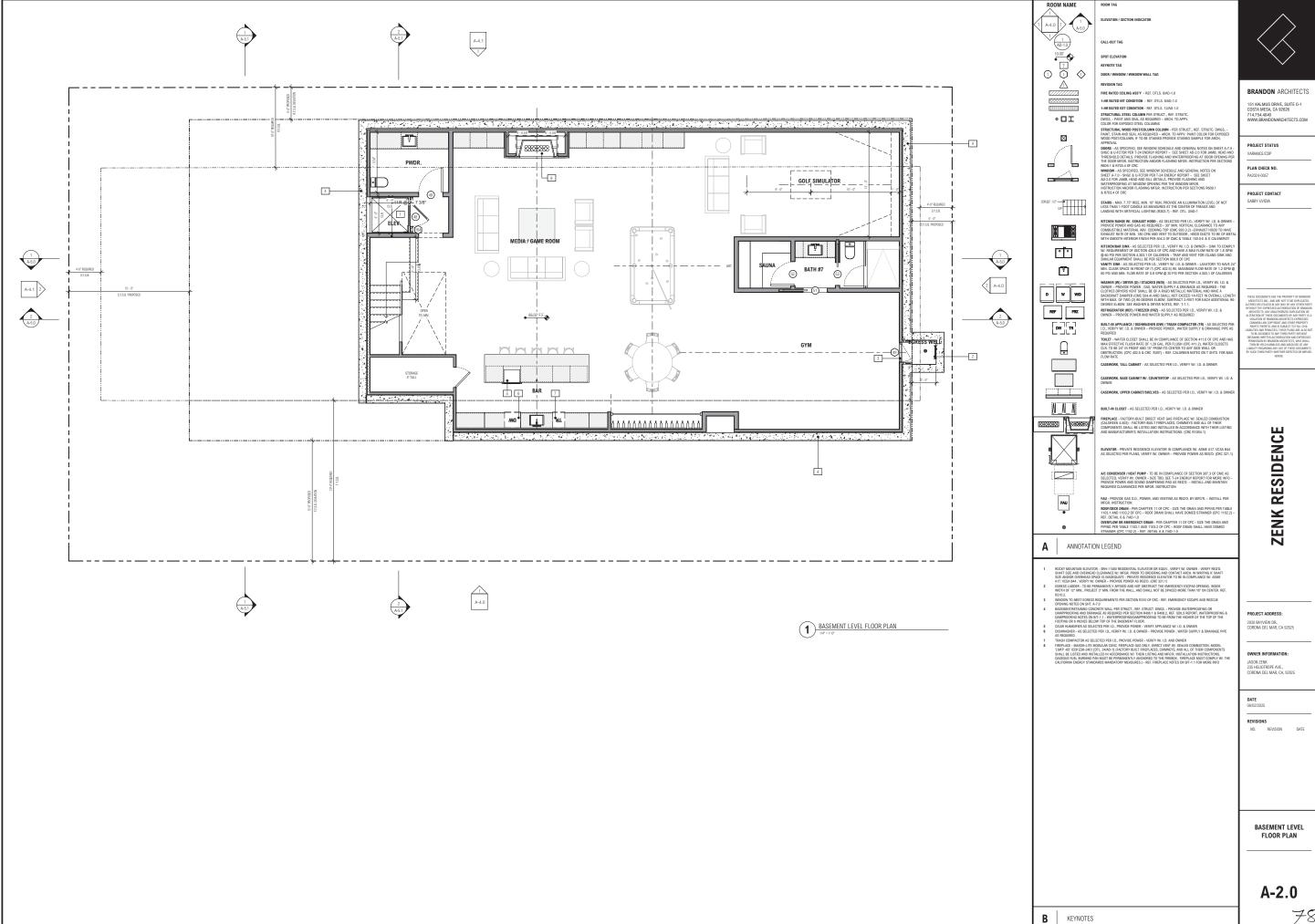
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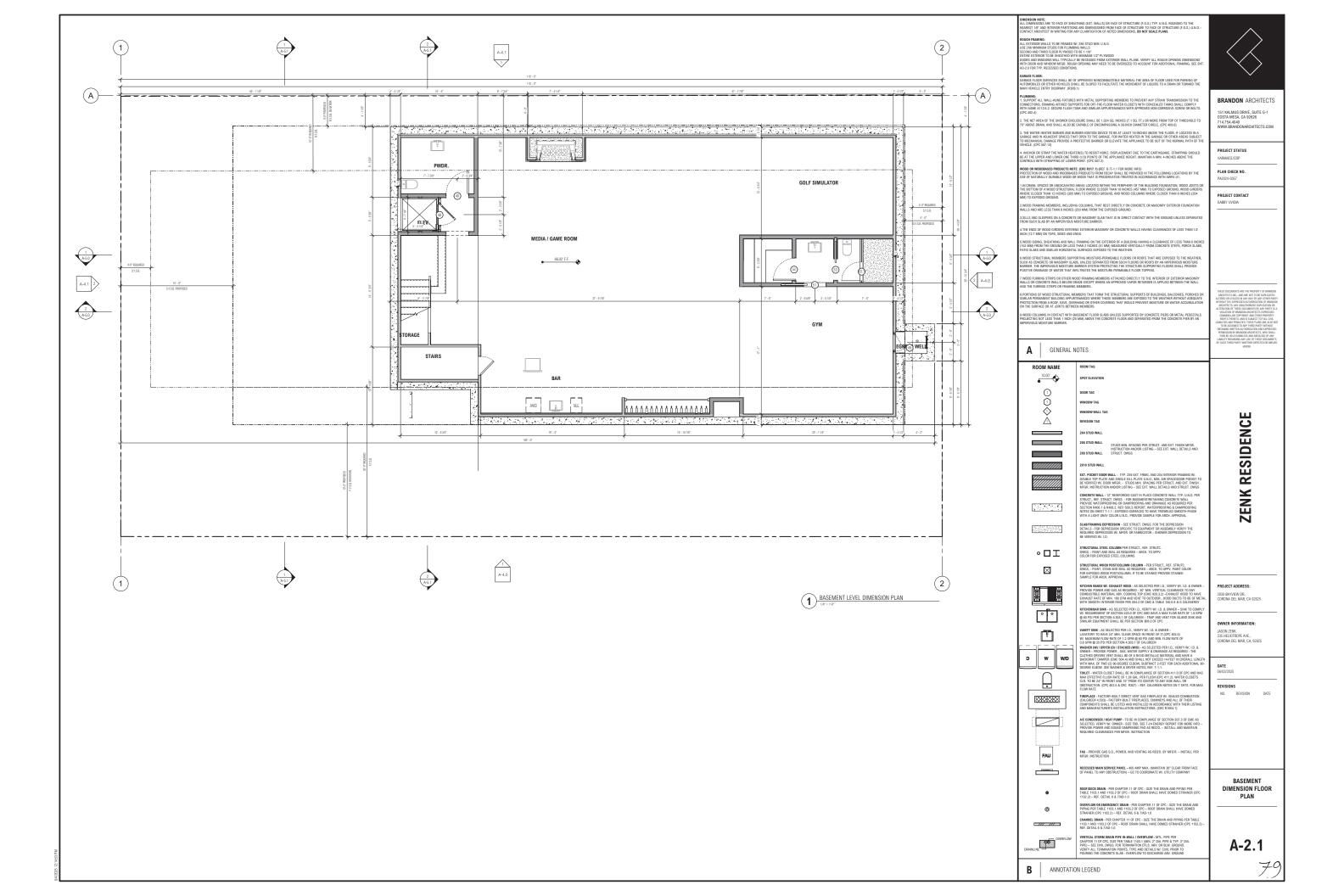
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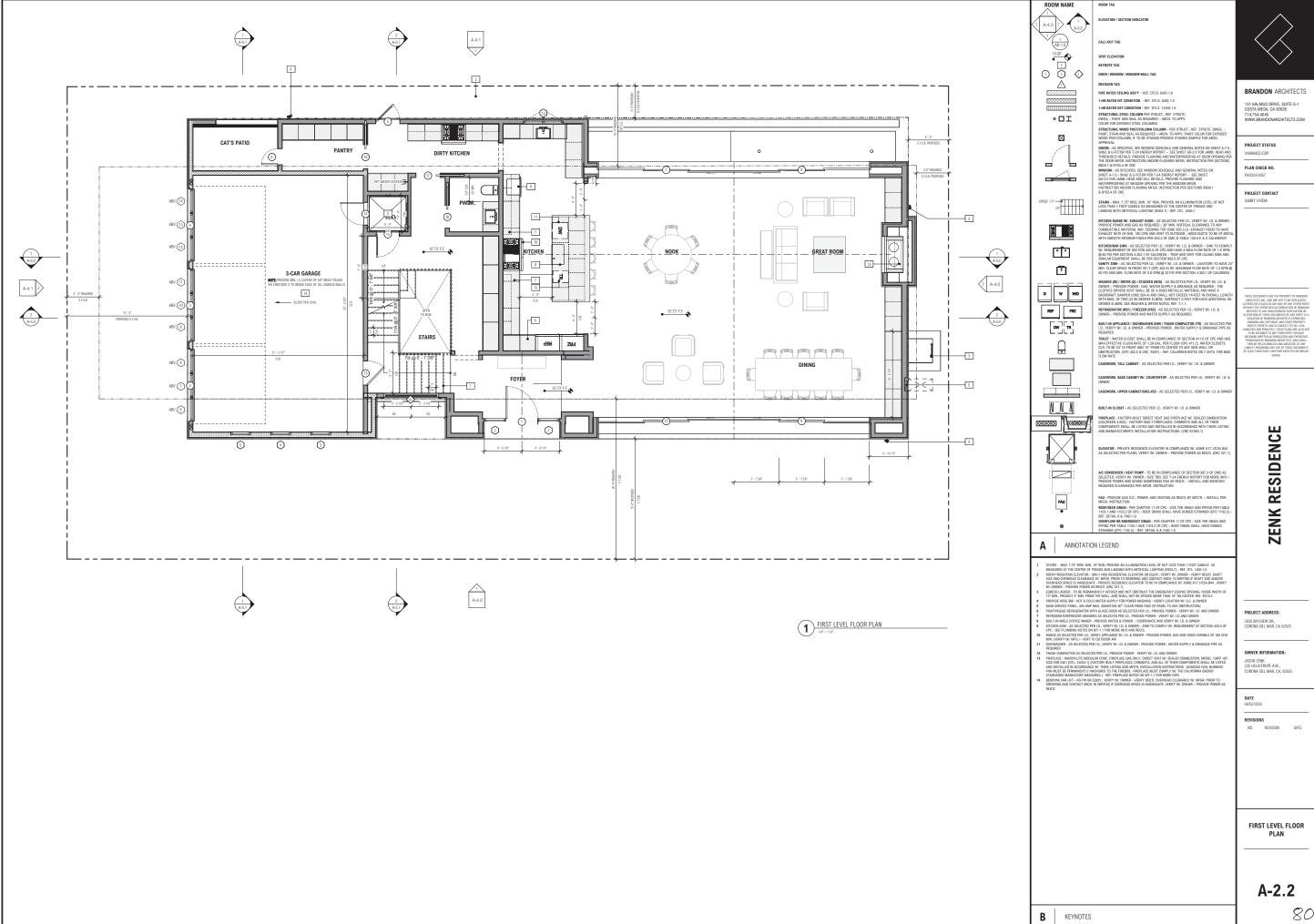
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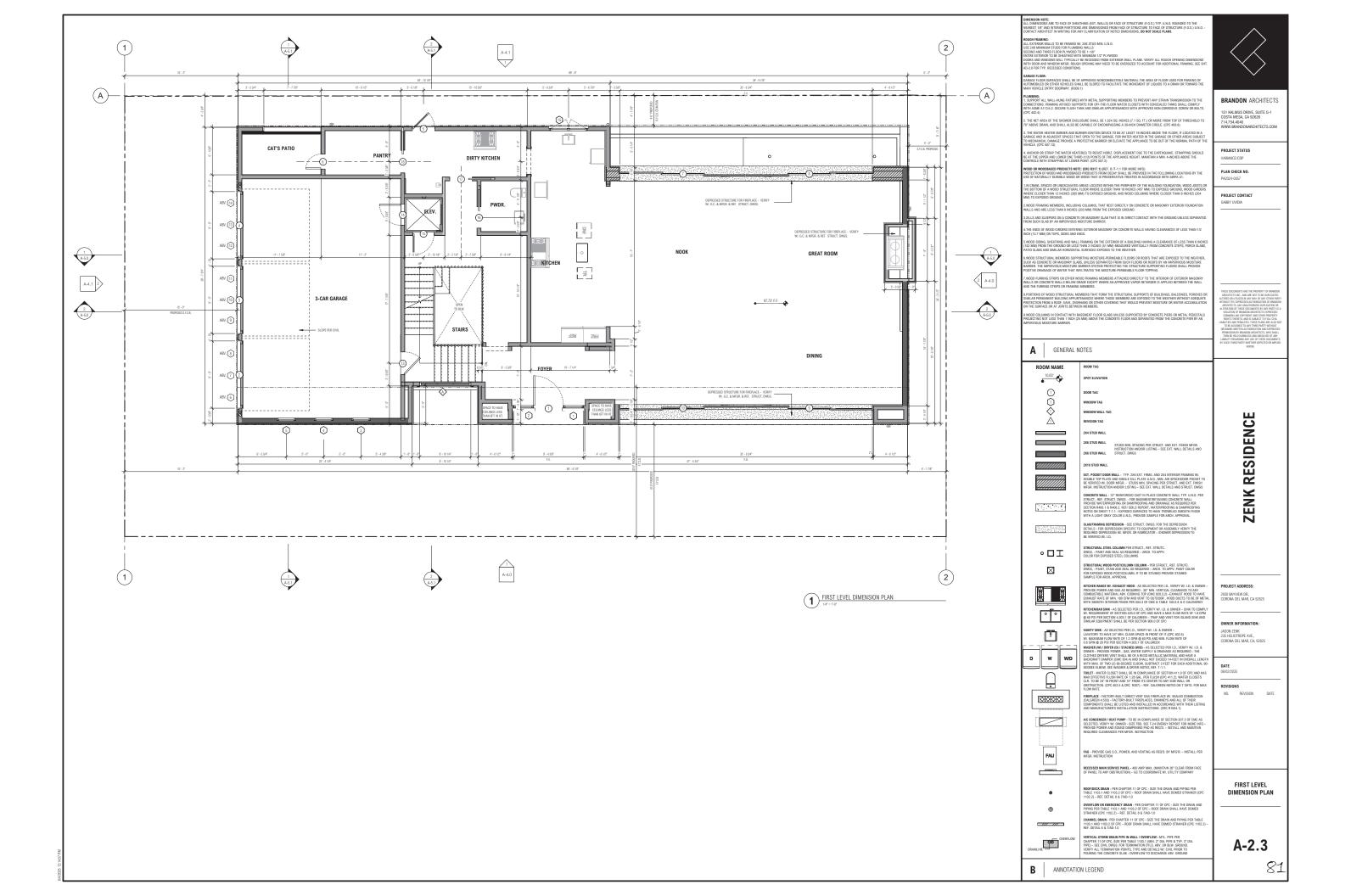
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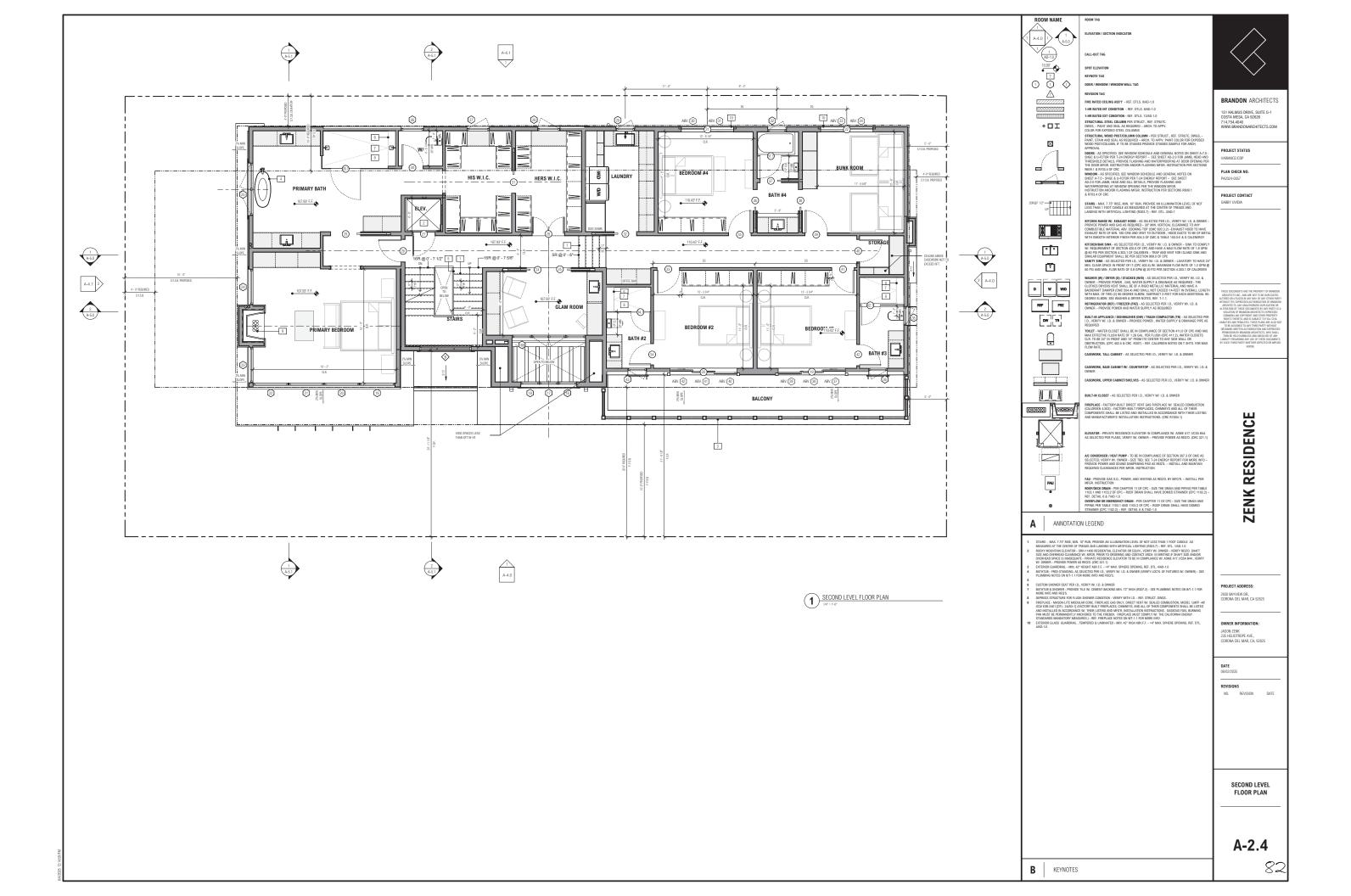
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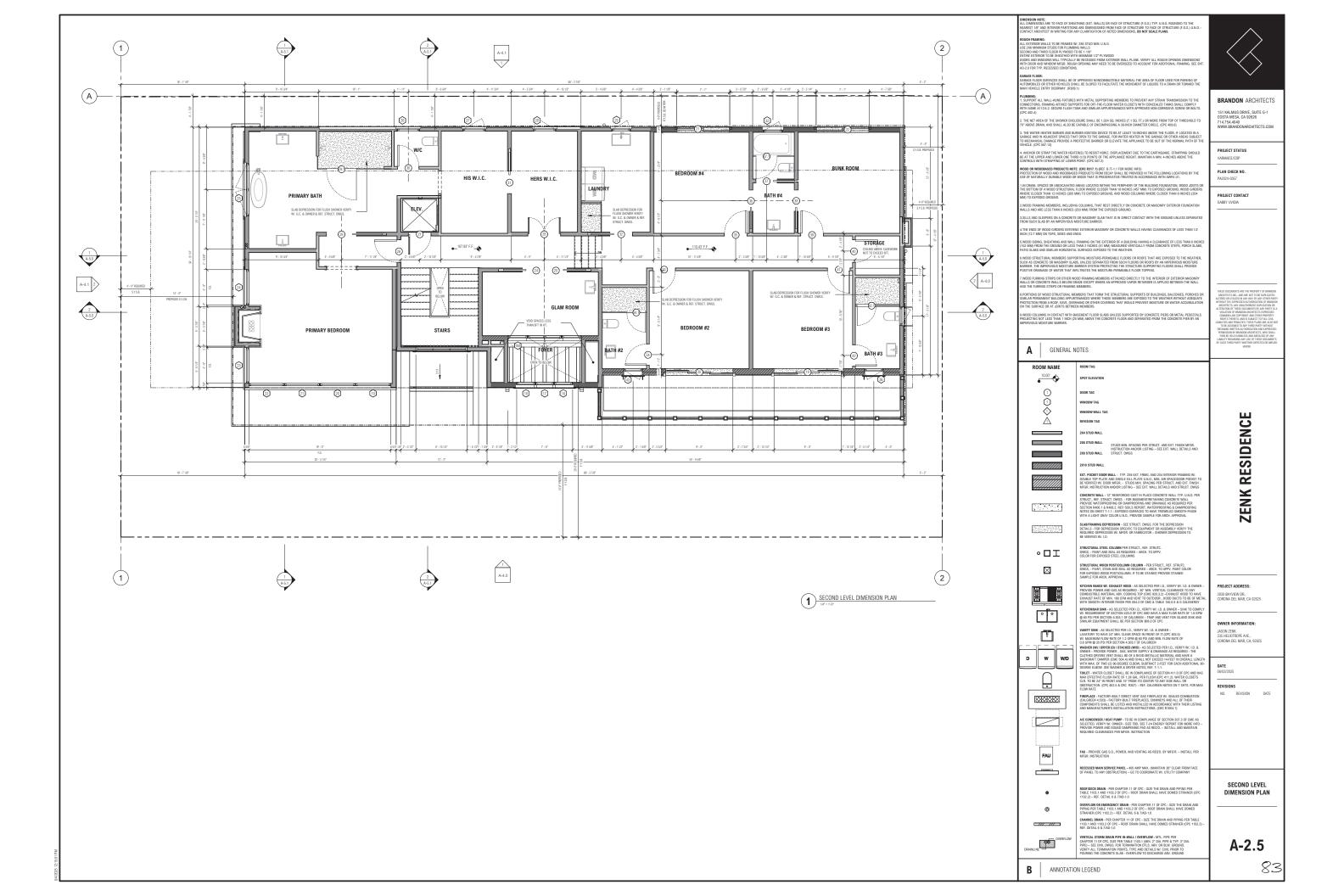


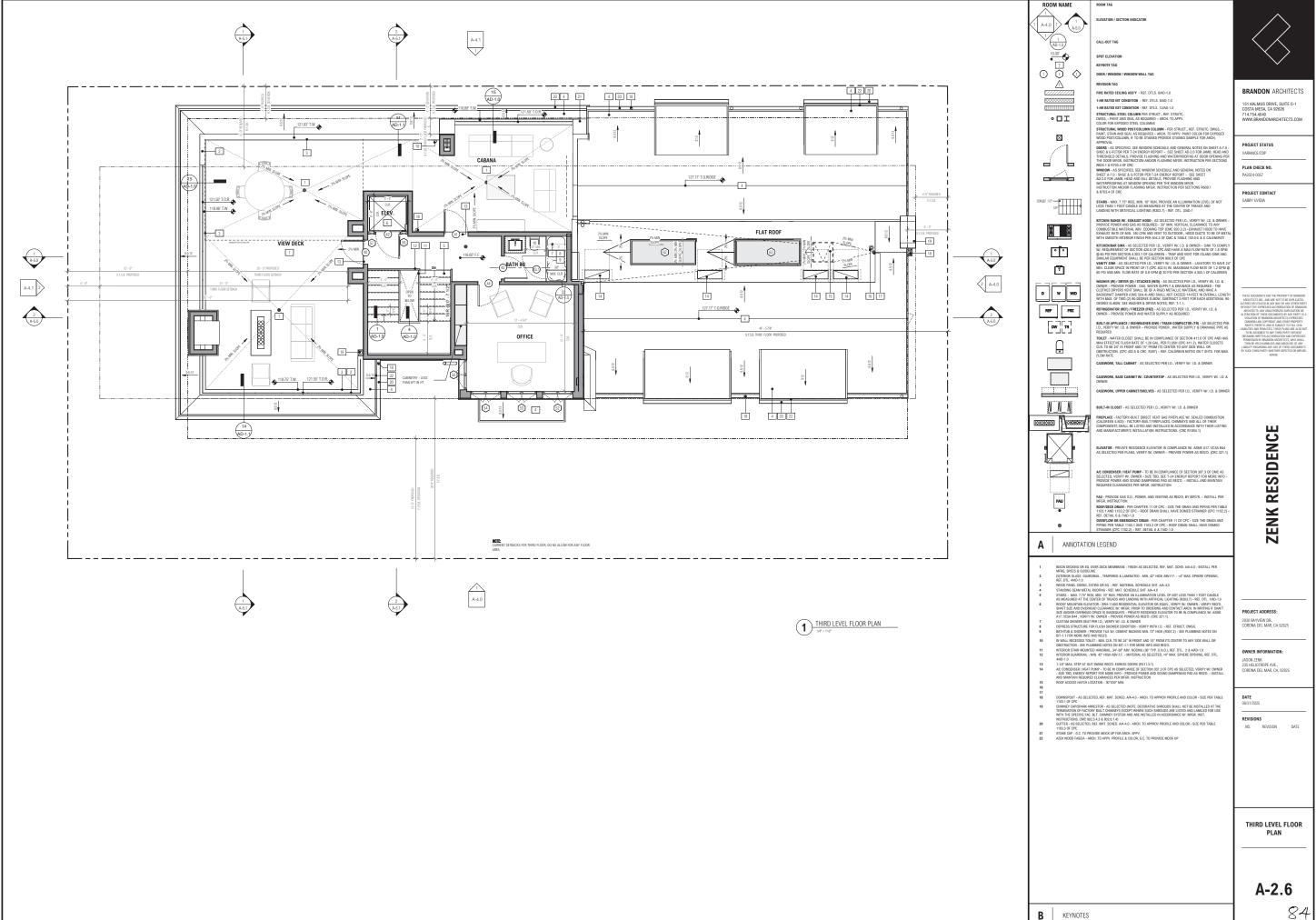


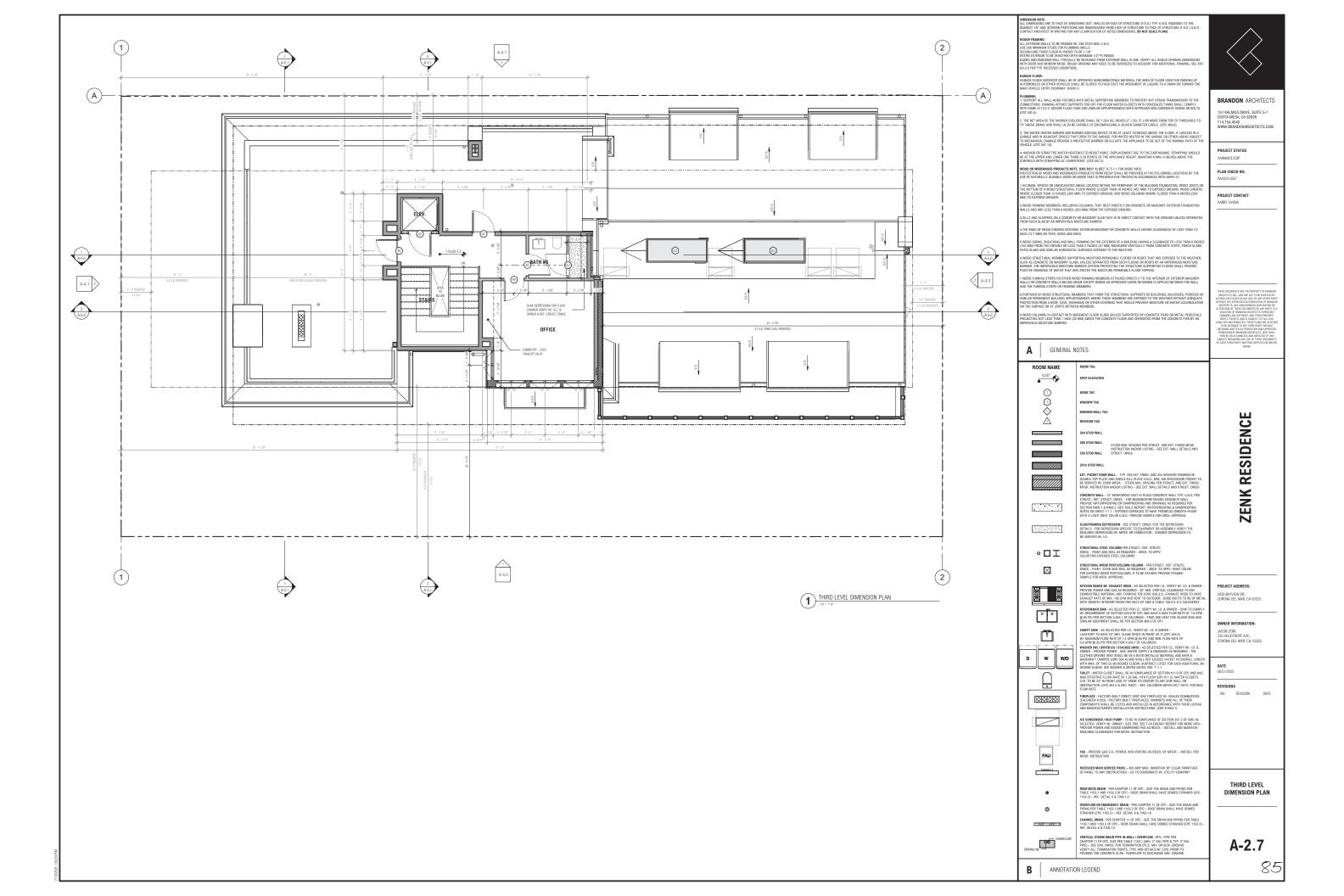












**BRANDON** ARCHITECTS

PROJECT STATUS

PLAN CHECK NO.

PROJECT CONTACT GABBY UVIDIA

RESIDENCE ZENK

PROJECT ADDRESS: 2830 BAYVIEW DR., CORONA DEL MAR, CA 92625

OWNER INFORMATION:

JASON ZENK 235 HELIOTROPE AVE., CORONA DEL MAR, CA, 92825

REVISIONS

ROOF PLAN

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